



















ALLJoist®

SPECIFIER GUIDE

INCLUDES AJS® 140 / 20 / 25







CCMC Report Number 12787-R ALLJOIST®



















High Performance Floor & Roof Systems



The SIMPLE FRAMING SYSTEM®



Architects, engineers, and designers trust Boise Cascade's engineered wood products to provide a better system for framing floors and roofs.



featuring beams, joists and rim boards that work together as a system, so you spend less time cutting and fitting. In fact, the SIMPLE FRAMING SYSTEM[®] uses fewer pieces and longer lengths than conventional framing, so you'll complete jobs in less time.

You'll Build Better Homes with the SIMPLE FRAMING SYSTEM[®]

Now it's easier than ever to design and build better floor systems. When you specify the SIMPLE FRAMING SYSTEM[®], your clients will have fewer problems with squeaky floors and ceiling gypsum board cracks. The SIMPLE FRAMING SYSTEM[®] also means overall better floor and roof framing than dimension lumber allows.

Better Framing Doesn't Have to Cost More

Boise Cascade Engineered Wood Products' SIMPLE FRAMING SYSTEM[®] often costs less than conventional framing methods when the resulting reduced labor and materials waste are considered. There's less sorting and cost associated with disposing of waste because you order only what you need. Although our longer lengths help your clients get the job done faster, they cost no more.

Environmentally Sound

As an added bonus, floor and roof systems built with AJS[®] Joists require about half the number of trees as those built with dimension lumber. This helps you design a home both you and future generations will be proud to own.

What Makes the SIMPLE FRAMING SYSTEM[®] So Simple?

☑ Floor and Roof Framing with ALLJOIST[®] Product

Light in weight, but heavy-duty, ALLJOIST[®] Product (AJS[®] Joists) have a better strength / weight ratio than dimension lumber. Knockouts can be removed for cross-ventilation and wiring.

☑ Ceilings Framed with AJS[®] Joists

The consistent size of AJS[®] Joists helps keep gypsum board flat and free of unsightly nail pops and ugly shadows, while keeping finish work to a minimum.

☑ VERSA-LAM[®] Beams for Floor and Roof Framing

These highly-stable beams are free of the large-scale defects that plague dimension beams. The result is quieter, flatter floors (no camber) and no shrinkage-related call-backs.

☑ Boise Cascade Rimboard

Boise Cascade Engineered Wood Products offer several engineered rimboard products regionally, including BC RIM BOARD® OSB, BC RIM BOARD® and VERSA-RIM® (check supplier or Boise Cascade EWP representative for availability). These products work with AJS® Joists to provide a solid connection at the critical floor/wall intersection.

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ALLJOIST[®] Architectural Specifications

Scope: This work includes the complete furnishing and installation of all AJS[®] Joists as shown on the drawings, herein specified and necessary to complete the work.

Materials: AJS[®] Joists shall be manufactured by Boise Cascade Engineered Wood Products with oriented strand board webs, machine stress rated (MSR) lumber flanges and waterproof, structural adhesives.

Joist webs shall be rated Structural I Exposure 1 by an agency listed by a model code evaluation service. The web panels shall be glued together to form a continuous web member. The web panels shall be machined to fit into a groove in the center of the wide face of the flange members so as to form a pressed glue joint at that junction.

Design: The AJS[®] Joists shall be sized and detailed to fit the dimensions and loads indicated on the plans. All designs shall be in accordance with allowable values and section properties developed in accordance with ASTM D5055, CSA O86, and listed under a CCMC product evaluation.

Drawing: Additional drawings showing layout and detail necessary for determining fit and placement in the building are (are not) to be provided by the supplier.

Fabrication: The AJS[®] Joists and section properties shall be manufactured in a plant evaluated for fabrication by the governing code evaluation service and under the supervision of a third-party inspection agency listed by the corresponding evaluation service.

Storage and Installation: The AJS[®] Joists, if stored prior to erection, shall be stored in a vertical and level position and protected from the weather. They shall be handled with care so they are not damaged.

The AJS[®] Joists are to be installed in accordance with the plans and the Boise Cascade Engineered Wood Products Installation Guide. Temporary construction loads which cause stresses beyond design limits are not



permitted. Erection bracing shall be provided to keep the AJS[®] Joists straight and plumb as required and to assure adequate lateral support for the individual AJS[®] Joists and the entire system until the sheathing material has been applied.

Codes: The AJS[®] Joists shall be evaluated by the CCMC evaluation service.

Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its BCI[®] Joist, VERSA-LAM[®], and ALLJOIST[®] products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed and used according to our Installation Guide.

For information about Boise Cascade's engineered wood products, including sales terms and conditions, warranties and disclaimers, visit our website at www.BC.com/ewp

BOISE CASCADE IS CERTIFIED BY SFI, AMERICA'S LEADING FORESTRY CERTIFICATION PROGRAM:

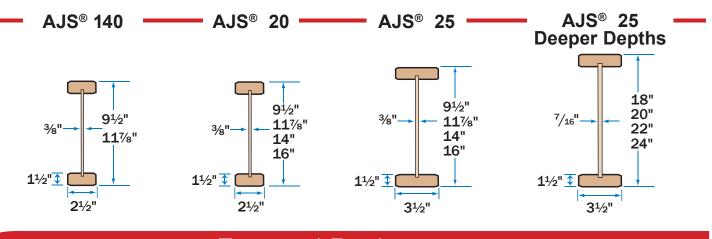
Boise Cascade doesn't own forests, but buys wood fiber in compliance with SFI, the Sustainable Forestry Initiative[®], which certifies the dominant share of North American forest acreage -- 160+ million acres. Boise Cascade is an SFI chain-of-custody certified national supplier. Chain-of-custody tracks and records possession and transfer of wood fiber from

forest of origin through all stages of distribution and production to the homebuilder. Chain-ofcustody assures that Boise Cascade products are made



using fiber from responsibly-managed forests and not from areas that are illegally harvested, major tropical wilderness areas or biodiversity hotspots. Boise Cascade's computerized chainof-custody system documents sourcing of all wood fiber purchased, ensuring that none gets into Boise Cascade inventory unless it comes from acceptable sources.

ALLJOIST[®] Product Profiles



Factored Resistances

Limit States Design (CANADA)

		Depth Resistance Resistance El Coefficient, I					Factored E Resistar	nd Bearing nce (Ibs)	Factored Intern Resista	nediate Bearing nce (Ibs)
AJS® Joist				Deformation	Joist Weight	1½" Min. Bea No Web Stiffeners	ring Length ⁽²⁾ WITH Web Stiffeners	3½" Min. Be No Web Stiffeners	aring Length WITH Web Stiffeners	
Series	[in]	[lbs-ft]	[lbs]	[x10 ⁶ lbs-in ²]	[x10 ⁶ lbs]	[lbs/ft]	[lbs]	[lbs]	[lbs]	[lbs]
AJS®	91⁄2	4095	1830	182	5.2	2.2	1500	1955	3705	3865
140	117⁄8	5305	2350	311	6.6	2.5	1505	2105	3770	4415
	9½	5675	1830	232	5.2	2.5	1500	1955	3705	3865
AJS®	117⁄8	7350	2350	394	6.6	2.8	1505	2105	3770	4415
20	14	8850	2825	579	7.8	3.0	1515	2240	3835	4940
	16	10265	3255	789	8.9	3.3	1530	2365	3890	5420
	9½	8935	1830	322	5.3	3.1	1500	1955	4100	4495
	117⁄8	11575	2350	545	6.7	3.4	1505	2105	4245	5035
	14	13940	2825	798	7.9	3.7	1515	2240	4370	5520
AJS®	16	16165	3255	1082	9.1	3.9	1530	2365	4495	5995
25	18 ⁽³⁾	18260	4750	1427	12.3	4.6		3535		7450
	20 ⁽³⁾	20405	5110	1813	13.7	4.9		3930		8065
	22 ⁽³⁾	22375	5475	2249	15.0	5.1		3930		8255
	24 ⁽³⁾	24325	5820	2738	16.5	5.4		3930		8435

NOTES:

- (1) All resistance factors, as per CSA O86 have been applied.
- (2) Minimum end bearing length is 1½" for 9½" to 16" depths and 1¾" for 18" and deeper.
- (3) AJS[®] Joists deeper than 16" require web stiffeners at all bearing locations.
- (4) The AJS[®] Joist deflection under uniform load may be calculated with the equation to the right:

BUILDING CODE EVALUATION REPORTS - CCMC Report Number 12787-R

ALLJOIST® Specifier Guide - CANADA

$$\Delta = \frac{5wl^4}{384 EI} + \frac{wl^2}{K}$$

 Δ = Deflection [in]

1

Κ

- w =Uniform load [lb/in]
 - = Centerline to centerline [in]
- *EI* = Stiffness value from table [lb-in²]
 - = Shear deflection factor from table [lb]

5

About Floor Performance

Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate end user to determine their expectation is critical. *Vibration* is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of the

floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to *increase the joist depth, limit joist deflections, glue and screw a thicker tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flange of the joists.*

					⁵⁄8" S	ubfloo	r (Nail	led)					
Live Load	d: 40 psf			Simple	e Span					Continuo	ous Span		
Dead Loa	ad: 15 psf		Bare Jois	t	Applied	l∕₂" Gypsur	m Ceiling		Bare Joist	:	Applied 1/2" Gypsum Ceiling		
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12" 16"		19.2"	12"	16"	19.2"
AJS®	9½	14'-7"	13'-7"	13'-1"	15'-0"	14'-0"	13'-5"	15'-10"	14'-9"	14'-2"	16'-3"	15'-2"	14'-7"
140	111⁄%	16'-5"	15'-4"	14'-9"	16'-11"	15'-9"	15'-2"	17'-10"	16'-8"	16'-0"	18'-5"	17'-2"	16'-5"
	91⁄2	15'-4"	14'-4"	13'-9"	15'-9"	14'-8"	14'-1"	16'-8"	15'-6"	14'-11"	17'-1"	15'-11"	15'-3"
AJS®	111/8	17'-4"	16'-2"	15'-6"	17'-9"	16'-6"	15'-10"	18'-11"	17'-6"	16'-10"	19'-7"	17'-11"	17'-3"
20	14	19'-1"	17'-7"	16'-10"	19'-8"	18'-1"	17'-4"	21'-2"	19'-4"	18'-5"	21'-10"	20'-0"	19'-0"
	16	20'-10"	19'-1"	18'-1"	21'-6"	19'-8"	18'-8"	23'-1"	21'-2"	20'-1"	23'-10"	21'-11"	20'-9"
	9½	16'-5"	15'-4"	14'-9"	16'-10"	15'-8"	15'-0"	17'-10"	16'-8"	16'-0"	18'-3"	17'-0"	16'-4"
	111/8	18'-8"	17'-3"	16'-7"	19'-2"	17'-8"	16'-11"	20'-8"	18'-11"	18'-0"	21'-3"	19'-6"	18'-6"
	14	20'-9"	19'-0"	18'-0"	21'-4"	19'-6"	18'-6"	23'-1"	21'-1"	20'-0"	23'-9"	21'-9"	20'-7"
AJS®	16	22'-8"	20'-8"	19'-8"	23'-4"	21'-4"	20'-3"	25'-2"	23'-0"	21'-10"	25'-11"	23'-9"	22'-6"
25	18	24'-9"	22'-7"	21'-5"	25'-5"	23'-3"	22'-1"	27'-4"	25'-1"	23'-9"	28'-2"	25'-10"	24'-6"
	20	26'-6"	24'-2"	22'-11"	27'-2"	24'-11"	23'-8"	29'-4"	26'-10"	25'-5"	30'-2"	27'-8"	26'-3"
	22	28'-2"	25'-9"	24'-5"	28'-11"	26'-6"	25'-2"	31'-2"	28'-6"	27'-1"	32'-1"	29'-5"	27'-11"
	24	29'-9"	27'-2"	25'-10"	30'-7"	28'-1"	26'-8"	33'-2"	30'-2"	28'-8"	34'-5"	31'-2"	29'-7"

					³∕₄" S	ubfloo	r (Nail	ed)						
Live Load	d: 40 psf			Simple	e Span					Continue	ous Span			
Dead Loa	ad: 15 psf		Bare Joist	t	Applied 1/2" Gypsum Ceiling				Bare Jois	t	Applied 1/2" Gypsum Ceiling			
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	
AJS®	9½	15'-3"	14'-3"	13'-7"	15'-7"	14'-7"	13'-11"	16'-6"	15'-5"	14'-9"	16'-11"	15'-10"	15'-2"	
140	111⁄8	17'-2"	16'-0"	15'-4"	17'-7"	16'-5"	15'-9"	18'-9"	17'-4"	16'-8"	19'-5"	17'-10"	17'-1"	
	91⁄2	16'-1"	15'-0"	14'-4"	16'-5"	15'-3"	14'-8"	17'-5"	16'-3"	15'-6"	17'-9"	16'-7"	15'-11"	
AJS®	111/8	18'-1"	16'-10"	16'-1"	18'-7"	17'-3"	16'-6"	20'-0"	18'-4"	17'-6"	20'-7"	18'-11"	17'-11"	
20	14	20'-2"	18'-5"	17'-7"	20'-9"	19'-0"	18'-0"	22'-4"	20'-5"	19'-4"	23'-0"	21'-1"	20'-0"	
	16	22'-1"	20'-2"	19'-0"	22'-8"	20'-9"	19'-8"	24'-5"	22'-4"	21'-2"	25'-1"	23'-1"	21'-10"	
	91⁄2	17'-2"	16'-0"	15'-4"	17'-6"	16'-4"	15'-7"	18'-10"	17'-5"	16'-8"	19'-3"	17'-9"	17'-0"	
	111/8	19'-9"	18'-0"	17'-3"	20'-2"	18'-5"	17'-7"	21'-10"	20'-0"	18'-11"	22'-5"	20'-6"	19'-5"	
	14	22'-0"	20'-1"	19'-0"	22'-6"	20'-7"	19'-6"	24'-4"	22'-3"	21'-1"	25'-0"	22'-11"	21'-8"	
AJS®	16	24'-0"	21'-11"	20'-8"	24'-7"	22'-6"	21'-3"	26'-7"	24'-4"	23'-0"	27'-3"	25'-0"	23'-8"	
25	18	26'-2"	23'-11"	22'-7"	26'-9"	24'-6"	23'-2"	28'-11"	26'-6"	25'-1"	29'-7"	27'-2"	25'-9"	
	20	28'-0"	25'-7"	24'-2"	28'-7"	26'-3"	24'-10"	31'-0"	28'-4"	26'-10"	31'-9"	29'-2"	27'-7"	
	22	29'-9"	27'-2"	25'-8"	30'-5"	27'-11"	26'-5"	33'-1"	30'-2"	28'-6"	34'-2"	31'-0"	29'-4"	
	24	31'-6"	28'-9"	27'-2"	32'-2"	29'-7"	28'-0"	35'-8"	31'-11"	30'-2"	36'-10"	32'-11"	31'-1"	

WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated

All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.10. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

NOTES:

- Tables are based on a uniform 40 psf live load and 15 psf dead load (Standard Term Load Duration).
- Floor tile will increase dead load and may require specific deflection limits.
- Minimum end bearing length is $1\frac{1}{2}$ " for $9\frac{1}{2}$ " to 16" depths and $1\frac{3}{4}$ " for 18" to 24".
- Stiffeners required at ALL bearing locations for all 18" to 24" deep joists (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured in between the supports (clearspan) and are based on uniformly loaded joists.
- Live load deflection is limited to L/360 and Total load deflection to L/240. Deflections are based on the bare joist stiffness.
- Spans shown are in accordance with NBCC2005: Part 9, and standard CAN-CSA O86-01.
- When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span.
 For other conditions, please contact your distributor or Boise Cascade EWP for assistance.
- It may be possible to exceed the limitations of these tables by analyzing a specific application with the Boise Cascade EWP's BC CALC[®] software and Boise Cascade WoodSizer software.
- The subfloor shall be CSA-rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP), or Douglas Fir Plywood (DFP).

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floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to *increase the joist depth, limit joist deflections, glue and screw a thicker tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flange of the joists.*

				⁵ ⁄8"	Subflo	or (Gl	ued &	Naile	d)				
Live Load	d: 40 psf			Simple	e Span					Continuo	ous Span		
Dead Loa	d: 15 psf		Bare Joist		Applied 3	∕₂" Gypsur	n Ceiling		Bare Joist		Applied 1/2" Gypsum Ceiling		
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"
AJS [®] 140	91⁄2	15'-7"	14'-9"	14'-3"	16'-0"	15'-2"	14'-8"	16'-10"	15'-11"	15'-5"	17'-4"	16'-5"	15'-11"
AJ3* 140	111⁄8	17'-6"	16'-6"	16'-0"	18'-0"	17'-0"	16'-5"	19'-2"	17'-10"	17'-3"	19'-11"	18'-6"	17'-10"
	91⁄2	16'-3"	15'-4"	14'-10"	16'-8"	15'-9"	15'-2"	17'-7"	16'-7"	16'-0"	18'-1"	17'-1"	16'-6"
AJS®	111⁄8	18'-3"	17'-2"	16'-7"	18'-10"	17'-8"	17'-0"	20'-2"	18'-9"	18'-0"	20'-11"	19'-5"	18'-7"
20	14	20'-3"	18'-10"	18'-0"	21'-0"	19'-6"	18'-8"	22'-6"	20'-10"	19'-11"	23'-3"	21'-7"	20'-8"
	16	22'-1"	20'-6"	19'-7"	22'-10"	21'-3"	20'-4"	24'-6"	22'-8"	21'-8"	25'-4"	23'-7"	22'-7"
	91⁄2	17'-3"	16'-2"	15'-8"	17'-7"	16'-7"	16'-0"	18'-10"	17'-7"	16'-11"	19'-4"	18'-0"	17'-4"
	111⁄8	19'-8"	18'-2"	17'-6"	20'-2"	18'-8"	17'-11"	21'-9"	20'-2"	19'-3"	22'-5"	20'-9"	19'-10"
	14	21'-10"	20'-2"	19'-3"	22'-6"	20'-10"	19'-10"	24'-2"	22'-4"	21'-4"	24'-11"	23'-1"	22'-1"
AJS®	16	23'-9"	21'-11"	21'-0"	24'-6"	22'-8"	21'-8"	26'-4"	24'-4"	23'-3"	27'-2"	25'-2"	24'-0"
25	18	25'-10"	23'-10"	22'-9"	26'-7"	24'-7"	23'-6"	28'-7"	26'-5"	25'-3"	29'-5"	27'-3"	26'-1"
	20	27'-7"	25'-6"	24'-4"	28'-5"	26'-4"	25'-2"	30'-7"	28'-3"	26'-11"	31'-6"	29'-2"	27'-11"
	22	29'-4"	27'-1"	25'-10"	30'-2"	27'-11"	26'-8"	32'-6"	30'-0"	28'-7"	33'-9"	31'-0"	29'-7"
	24	31'-0"	28'-7"	27'-4"	31'-11"	29'-7"	28'-3"	34'-11"	31'-8"	30'-3"	36'-4"	32'-10"	31'-4"

				³ /4"	Subflo	or (Gl	ued &	Naile	d)				
Live Load	d: 40 psf			Simple	e Span					Continuo	ous Span		
Dead Loa	d: 15 psf		Bare Joist	t	Applied	1⁄2" Gypsur	n Ceiling		Bare Joist	i	Applied 1/2" Gypsum Ceiling		
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"
AJS [®] 140	91⁄2	16'-7"	15'-7"	15'-1"	17'-0"	16'-0"	15'-6"	17'-11"	16'-11"	15'-10"	18'-6"	17'-5"	15'-10"
AJ3- 140	111⁄8	18'-8"	17'-6"	16'-10"	19'-3"	18'-0"	17'-4"	20'-7"	19'-2"	18'-3"	21'-4"	19'-11"	18'-1"
	91⁄2	17'-3"	16'-3"	15'-8"	17'-7"	16'-7"	16'-0"	18'-9"	17'-7"	16'-11"	19'-4"	18'-0"	17'-4"
AJS®	111⁄8	19'-7"	18'-2"	17'-6"	20'-2"	18'-9"	17'-11"	21'-8"	20'-1"	19'-2"	22'-4"	20'-10"	19'-10"
20	14	21'-10"	20'-2"	19'-3"	22'-5"	20'-10"	19'-11"	24'-1"	22'-4"	21'-4"	24'-10"	23'-1"	22'-1"
	16	23'-9"	22'-0"	20'-11"	24'-5"	22'-9"	21'-8"	26'-3"	24'-4"	23'-2"	27'-1"	25'-2"	24'-1"
	91⁄2	18'-3"	17'-2"	16'-6"	18'-8"	17'-6"	16'-10"	20'-2"	18'-8"	17'-11"	20'-8"	19'-2"	18'-4"
	111⁄8	21'-1"	19'-6"	18'-7"	21'-7"	20'-0"	19'-1"	23'-4"	21'-7"	20'-7"	23'-11"	22'-2"	21'-2"
	14	23'-5"	21'-8"	20'-7"	24'-0"	22'-3"	21'-2"	25'-11"	23'-11"	22'-10"	26'-7"	24'-8"	23'-6"
AJS®	16	25'-6"	23'-6"	22'-5"	26'-1"	24'-2"	23'-1"	28'-2"	26'-1"	24'-8"	28'-11"	26'-10"	24'-8"
25	18	27'-7"	25'-6"	24'-4"	28'-4"	26'-3"	25'-0"	30'-7"	28'-3"	26'-11"	31'-4"	29'-1"	27'-9"
	20	29'-6"	27'-3"	26'-0"	30'-3"	28'-1"	26'-9"	32'-8"	30'-2"	28'-9"	33'-10"	31'-1"	29'-8"
	22	31'-4"	29'-0"	27'-7"	32'-2"	29'-10"	28'-5"	35'-5"	32'-1"	30'-6"	36'-7"	33'-2"	31'-6"
	24	33'-4"	30'-7"	29'-1"	34'-5"	31'-6"	30'-0"	38'-1"	34'-4"	32'-3"	39'-5"	35'-9"	33'-6"

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NOTES:

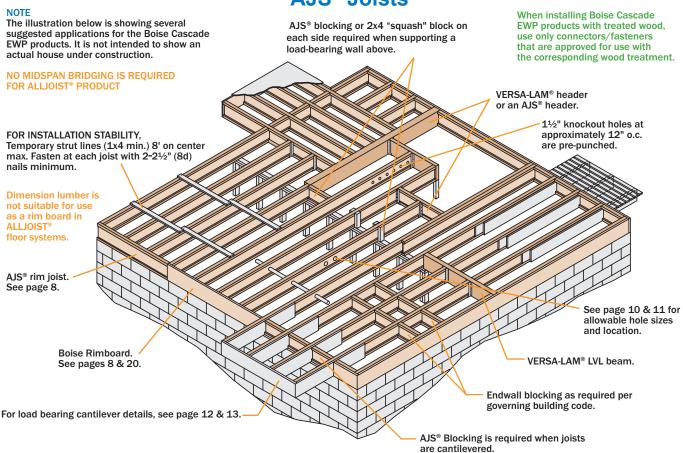
- Tables are based on a uniform 40 psf live load and 15 psf dead load (Standard Term Load Duration).
- Floor tile will increase dead load and may require specific deflection limits.
- Minimum end bearing length is 1½" for 9½" to 16" depths and 1¾" for 18" to 24".
- Stiffeners required at ALL bearing locations for all 18" to 24" deep joists (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured in **between the supports** (clearspan) and are based on uniformly loaded joists.
- Live load deflection is limited to L/360 and Total load deflection to L/240. Deflections are based on the bare joist stiffness.

Spans shown are in accordance with NBCC2005: Part 9, and standard CAN-CSA 086-01.

- When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span. For other conditions, please contact your distributor or Boise Cascade EWP, for assistance.
- It may be possible to exceed the limitations of these tables by analyzing a specific application with the Boise Cascade BC CALC® software and Boise Cascade WoodSizer software.
- The subfloor shall be CSA rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP), or Douglas Fir Plywood (DFP).
- Subfloor adhesive shall comply with CGSB standard CAN-CGSB 71.26-M88 "Adhesives for Field-gluing Plywood to Lumber Framing for Floor Systems" or APA Performance Specification AFG-01.

Floor Framing

AJS® Joists



BCI® Joists, VERSA-LAM® and ALLJOIST® must be stored, installed and used in accordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. VERSA-LAM®, ALLJOIST®, and BCI® Joists must be wrapped, covered, and stored off of the ground on stickers at all times prior to installation. VERSA-LAM®, ALLJOIST® and BCI® Joists are intended only for applications that assure no exposure

SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS[®] JOISTS UNTIL ALL HANGERS, AJS[®] RIM JOISTS, RIM BOARDS, AJS[®] BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW. SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS[®] Joist and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS[®] Joist at the end of the bay.
- All hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels, and x-bracing must be completely installed and properly nailed as each AJS[®] Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS[®] Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each AJS[®] Joist with 2-2¹/₂" (8d) nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS[®] Joist to within ¹/₂ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.
- Do not stack construction materials (sheathing, drywall, etc) in the middle of AJS[®] Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.

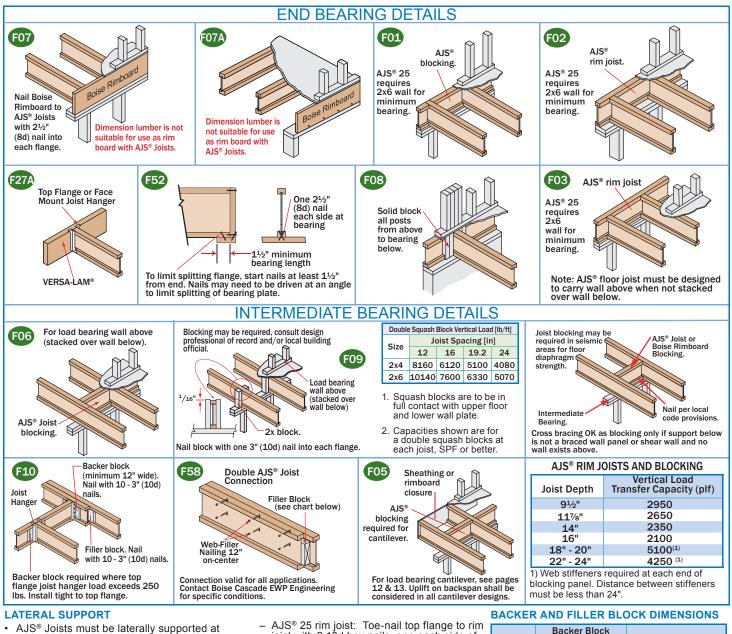
ALLJOIST[®] Specifier Guide - CANADA

to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install VERSA-LAM®, ALLJOIST®, and BCI® Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.



Floor Framing Details

Additional floor framing details available with BC FRAMER® software



- AJS[®] Joists must be laterally supported at the ends with hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels or x-bracing. AJS[®] blocking panels or x-bracing are required at cantilever supports. Blocking may be required at intermediate bearings for floor diaphragm as per Code, consult local building official
- consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOISTS

- $1\frac{1}{2}$ inches is required at end supports (1³/₄ inches for 18" to 24" deep). 3¹/₂ inches is required at cantilever and intermediate supports
- Longer bearing lengths allow higher reaction values. Refer to the building Code evaluation report or the BC CALC® software.

NAILING REQUIREMENTS

- AJS® rim joist, rim board or closure panel to AJS[®] Joist:
 - Rims or closure panel 11/4 inches thick and less:

2-21/2" (8d) nails, one each in the top and bottom flange. AJS® 140/20 rim joist: 2-31/2" (16d) box

nails, one each in the top and bottom flange.

- AJS[®] 25 rim joist: Toe-nail top flange to rim joist with 2-10d box nails, one each side of flange.
- AJS® rim joist, rim board or AJS® blocking panel to support
- 21/2" (8d) nails at 6 inches on center.
- When used for shear transfer, follow the building designer's specification.
- AJS[®] Joist to support:
 - 2-2½" (8d) nails, one on each side of the web, placed 1½ inches minimum from the end of the AJS[®] Joist to limit splitting.
- Sheathing to AJS[®] Joist:
- Prescriptive residential floor sheathing nailing requires 2½" (8d) common nails @ 6" o.c. on edges and @ 12" o.c. in the field as per Code.
- Maximum nail spacing for minimum lateral stability = 24".
- 14 gauge staples may be substituted for 21/2" (8d) nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

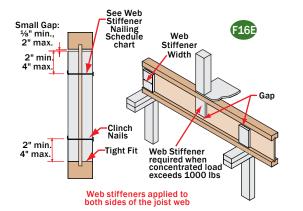
Series	Backer Block Thickness	Filler Block Thickness
AJS® 140	11/8" or two 1/2" wood panels	2 x + 5%" wood panel
AJS [®] 20	1 ¹ ⁄/s" or two ¹ ⁄2" wood panels	2 x + 5%" wood panel
AJS [®] 25	2 x _ lumber	Double 2 x lumber

- Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced fit.
- For deeper AJS® 25 Joists, stack 2x lumber or use multiple pieces of 3/4" wood panels.

WEB STIFFENER REQUIREMENTS

- See Web Stiffener Requirements on page 9.
- PROTECT AJS® JOISTS FROM THE WEATHER
- AJS® Joists is intended only for applications that provide permanent protection from the weather. Bundles of product should be covered and stored off of the ground on stickers

Web Stiffener Requirements



W	Web Stiffener Specifications													
Series	For Structral Capacity (Min. Thick)	Lateral Restraint in Hanger	Minimum Width											

1"

2x4 lumber (vertical)

11/2"

AJS®

140/20

AJS[®] 25

W	eb Stiffener Nailing Schedule								
ALLJOIST [®] Series	Joist Depth	Nailing							
AJS [®] 140 / 20 / 25	9½" – 11%"	3-3" (10d)							
AJS* 1407 207 25	14" – 24"	5-3" (10d)							

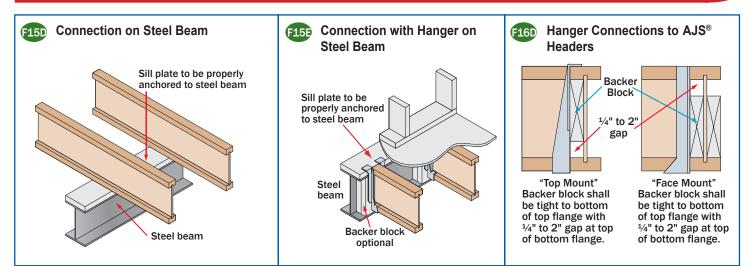
NOTES:

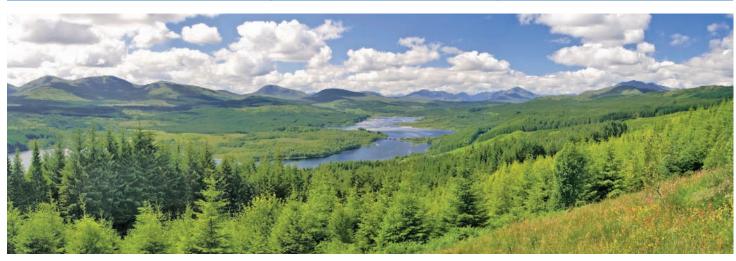
2⁵/₁₆"

Web stiffeners are optional except as noted below:

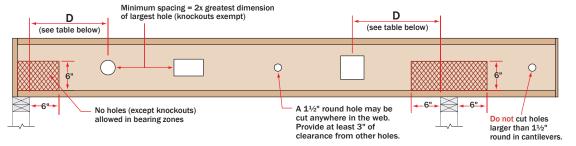
- Stiffeners required at ALL bearing locations for all 18" to 24" deep joists.
- Web stiffeners are always required in hangers that do not extend up to support the top flange of the AJS[®] Joist. Web stiffeners may be required with certain sloped or skewed hangers or to achieve uplift values. Refer to the hanger manufacturer's installation requirements.
- Web stiffeners may be cut from structural rated wood panels, engineered rimboard or 2x lumber (AJS[®] 25 only).
- For Structural Capacity: Web stiffeners needed to increase the AJS[®] Joist's reaction capacity at a specific bearing location.
- Web stiffeners are always required in certain roof applications. See Roof Framing Details on page 15.
- Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- Web stiffeners may be used to increase allowable reaction values. See Factored Resistances Limit States Design (CANADA) on page 4 of this guide or the BC CALC[®] software.

Connection Details





¹⁰ Hole Cutting Charts for Residential Applications (40/15)



AJS[®] Joists are manufactured with 1¹/₂" round perforated knockouts in the web at approximately 12" on center Minimum distance from support, listed in table below, is required for all holes greater than 1¹/₂"

1' - 0" 1' 10' 1' - 0" 2' 12' 1' - 0" 4' 14' 1' - 0" 5' 16' 2' - 0" 6' 18' 3' - 0" 7'	9½" 6" 9" 1' - 6" - 2' - 6" - 4' - 0" -		ace of a 3" 1' - 0" 1' - 0"	any sup 113 6" 1' - 0"		the cer		of hole 14		OIST D	EPTH	• HOLE		[IN]
ift] 3" 8' 1'-0" 1' 10' 1'-0" 2' 12' 1'-0" 4' 14' 1'-0" 5' 16' 2'-0" 6' 18' 3'-0" 7'	6" 9' 1' - 6'' - 2' - 6'' - 4' - 0'' -	-	1' - 0''	6"		12"		14	4"			16	5"	
1' - 0" 1' 10' 1' - 0" 2' 12' 1' - 0" 4' 14' 1' - 0" 5' 16' 2' - 0" 6' 18' 3' - 0" 7'	1' - 6'' - 2' - 6'' - 1' - 0'' -	-	1' - 0''	U U	9"	12"							-	
10' 1' - 0'' 2' 12' 1' - 0'' 4' 14' 1' - 0'' 5' 16' 2' - 0'' 6' 18' 3' - 0'' 7'	2' - 6'' - 1' - 0'' -	-		1' - 0''		12	3"	6"	9"	12"	3"	6"	9"	12"
12' 1' - 0'' 4' 14' 1' - 0'' 5' 16' 2' - 0'' 6' 18' 3' - 0'' 7'	1' - 0'' -		1' - 0''		-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''
14' 1' - 0'' 5' 16' 2' - 0'' 6' 18' 3' - 0'' 7'			1 - 0	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''
16' 2' - 0'' 6' 18' 3' - 0'' 7'		-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 6''
18' 3'-0'' 7'	5' - 0'' -	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 6''	-	1' - 0''	1' - 0''	1' - 0''	2' - 6''
	6' - 6'' -	-	1' - 0''	2' - 0''	-	-	1' - 0''	1' - 0''	2' - 6''	-	1' - 0''	1' - 0''	1' - 0''	3' - 6''
20' 4'-0" 9'	7' - 6'' -	-	1' - 0''	3' - 6''	-	-	1' - 0''	1' - 0''	4' - 0''	-	1' - 0''	1' - 0''	1' - 0''	4' - 6''
	9' - 0'' -	-	1' - 0''	4' - 6''	-	-	1' - 0''	1' - 0''	5' - 0''	-	1' - 0''	1' - 0''	2' - 0''	6' - 0''
22' 5'-0'' 10	0' - 0'' -	-	1' - 6''	5' - 6''	-	-	1' - 0''	2' - 6''	6' - 0''	-	1' - 0''	1' - 0''	3' - 0''	7' - 0''
24' 6' - 6'' 11	1' - 6'' -	-	2' - 6''	7' - 0''	-	-	1' - 0''	3' - 6"	7' - 6''	-	1' - 0''	1' - 0''	4' - 0''	8' - 0''
26' -		-	4' - 0''	8' - 0''	-	-	1' - 0''	4' - 6''	8' - 6''	-	1' - 0''	1' - 6''	5' - 6''	9' - 6''
28' -		-	5' - 0''	9' - 0''	-	-	2' - 0''	5' - 6''	10' - 0''	-	1' - 0''	2' - 6''	6' - 6''	10' - 6''
30' -		-	-	-	-	-	3' - 0''	7' - 0''	11' - 0''	-	1' - 0''	4' - 0''	7' - 6''	12' - 0''
32' -		-	-	-	-	-	4' - 0''	8' - 0''	12' - 6''	-	1' - 6''	5' - 0''	9' - 0''	13' - 0''
34' -		-	-	-	-	-	-	-	-	-	2' - 6''	6' - 0''	10' - 0''	14' - 6''

SQUARE HOLES

Mir	Minimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]															[IN]
Span		91⁄	⁄2"			11	7/8"			14	4"		16"			
[ft]	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"	3"	6"	9"	12"
8'	1' - 0''	1' - 6''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	1' - 0''
10'	1' - 0''	2' - 6''	-	-	1' - 0''	1' - 0''	-	-	1' - 0''	1' - 0''	1' - 0''	-	1' - 0''	1' - 0''	1' - 0''	2' - 6''
12'	1' - 6''	4' - 0''	-	-	1' - 0''	1' - 6''	-	-	1' - 0''	1' - 0''	2' - 6''	-	1' - 0''	1' - 0''	1' - 0''	3' - 6''
14'	2' - 6''	5' - 0''	-	-	1' - 0''	2' - 6''	-	-	1' - 0''	1' - 0''	3' - 6''	-	1' - 0''	1' - 0''	2' - 0''	4' - 6''
16'	3' - 6''	6' - 6''	-	-	1' - 6''	4' - 0''	-	-	1' - 0''	2' - 0''	4' - 6''	-	1' - 0''	1' - 0''	3' - 0''	6' - 0''
18'	5' - 0''	7' - 6''	-	-	2' - 6''	5' - 0''	-	-	1' - 0''	3' - 0''	6' - 0''	-	1' - 0''	1' - 6''	4' - 0''	7' - 0''
20'	6' - 0''	9' - 0''	-	-	3' - 6''	6' - 6''	-	-	2' - 0''	4' - 6''	7' - 0''	-	1' - 0''	2' - 6''	5' - 6''	8' - 6''
22'	7' - 0''	10' - 0''	-	-	5' - 0''	7' - 6''	-	-	3' - 0''	5' - 6''	8' - 6''	-	1' - 0''	3' - 6''	6' - 6''	10' - 0''
24'	8' - 6''	11' - 6''	-	-	6' - 0''	8' - 6''	-	-	4' - 0''	6' - 6''	9' - 6''	-	2' - 0''	5' - 0''	7' - 6''	11' - 0''
26'	-	-	-	-	7' - 0''	10' - 0''	-	-	5' - 0''	8' - 0''	11' - 0''	-	3' - 6''	6' - 0''	9' - 0''	12' - 6''
28'	-	-	-	-	8' - 6''	11' - 0''	-	-	6' - 0''	9' - 0''	12' - 0''	-	4' - 6''	7' - 0''	10' - 0''	13' - 6''
30'	-	-	-	-	-	-	-	-	7' - 6''	10' - 0''	13' - 6''	-	5' - 6''	8' - 6''	11' - 6''	*
32'	-	-	-	-	-	-	-	-	8' - 6''	11' - 6''	14' - 6''	-	6' - 6''	9' - 6''	12' - 6''	*
34'	-	-	-	-	-	-	-	-	-	-	-	-	8' - 0''	11' - 0''	14' - 0''	*

TABLE 3

TABLE 2

RECTANGULAR HOLES

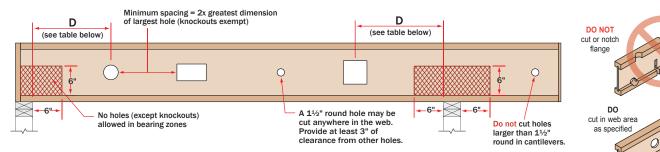
Mir	inimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]															
Span		91/	2"			11	7/8"			14	4"			16	6"	
[ft]	5"x8"	5"x10"	5"x12"	5"x14"	7"x10"	7"x12"	7"x14"	7"x16"	10"x12"	10"x14"	10"x16"	10"x18"	10''x16''	10''x18''	12''x14''	12''x16''
8'	1' - 6"	2' - 0"	2' - 0"	2' - 6"	1' - 0''	1' - 6''	2' - 0''	2' - 6''	1' - 6''	2' - 6''	3' - 0''	*	1' - 6''	2' - 6''	2' - 0''	3' - 0''
10'	2' - 6"	3' - 0"	3' - 6"	4' - 0"	2' - 0''	2' - 6''	3' - 6''	4' - 0''	3' - 0''	3' - 6''	4' - 6''	*	3' - 0''	4' - 0''	3' - 0''	4' - 0''
12'	3' - 6"	4' - 0''	4' - 6"	5' - 0"	3' - 6''	4' - 0''	4' - 6''	5' - 0''	4' - 0''	4' - 6''	5' - 6''	*	4' - 0''	5' - 0''	4' - 6''	5' - 6''
14'	5' - 0"	5' - 6"	6' - 0"	6' - 6"	4' - 6''	5' - 0''	6' - 0''	6' - 6''	5' - 0''	6' - 0''	*	*	5' - 6''	6' - 6''	5' - 6''	6' - 6''
16'	6' - 0"	6' - 6"	7' - 0"	7' - 6"	5' - 6''	6' - 6''	7' - 0''	*	6' - 6''	7' - 6''	*	*	6' - 6''	7' - 6''	7' - 0''	*
18'	7' - 6"	8' - 0"	8' - 6"	*	7' - 0''	7' - 6''	8' - 6''	*	7' - 6''	8' - 6''	*	*	8' - 0''	*	8' - 0''	*
20'	8' - 6"	9' - 0''	9' - 6"	*	8' - 0''	9' - 0''	9' - 6''	*	9' - 0''	*	*	*	9' - 0''	*	9' - 6''	*
22'	10' - 0"	10' - 6"	*	*	9' - 6''	10' - 0''	*	*	10' - 6''	*	*	*	10' - 6''	*	*	*
24'	11' - 0"	*	*	*	10' - 6''	11' - 6''	*	*	11' - 6''	*	*	*	11' - 6''	*	*	*
26'	-	-	-	-	12' - 0''	*	*	*	*	*	*	*	*	*	*	*
28'	-	-	-	-	13' - 6''	*	*	*	*	*	*	*	*	*	*	*
30'	-	-	-	-	-	-	-	-	*	*	*	*	*	*	*	*
32'	-	-	-	-	-	-	-	-	*	*	*	*	*	*	*	*
34'	-	-	-	-	-	-	-	-	-	-	-	-	*	*	*	*

DO NOT cut or notch flange DO cut in web area as specified O

NOTES:

- 1. Hole may be positioned vertically anywhere in the web.
- Tables 1-6 are for uniformly loaded maximum loads of 40 psf live loads and 15 psf dead loads on simple span application.
- 3. For other load conditions or hole sizes, contact your local distributor.
- It may be possible to exceed the limitations of those tables by analysing a specific situation with the BC CALC[®] Software.
- * = Holes may be acceptable, contact your local distributor.

Hole Cutting Charts for Residential Applications (40/15) 11

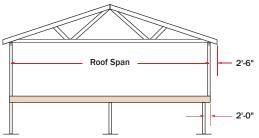


AJS® Joists are manufactured with 11/2" round perforated knockouts in the web at approximately 12" on center

Minimum distance from support, listed in table below, is required for all holes greater than 1 ¹ / ₂ "																	
TABI	LE 4						F	NOS	ND H	OLE	S						
Mir	Minimum distance from inside face of any support to the centerline of hole JOIST DEPTH • HOLE SIZE [IN]											NOTES:					
Span		18	8"			2	0"			2	2"			2	4"		1. Hole may vertically
[ft]	3''	6''	9''	12''	6"	9"	12''	15''	6''	9"	12''	15''	9''	12"	15''	18''	the web.
8' 10'	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	2' - 0'' 3' - 6''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	2. Tables 1-
12'	1'-0''	1' - 0''	1' - 0''	2'-6"	1'-0"	1'-0''	1'-0"	4' - 6''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 0''	2' - 0"	uniformly
14'	1' - 0''	1' - 0''	1' - 0''	3' - 6''	1' - 0''	1' - 0''	1' - 0''	6' - 0''	1' - 0''	1' - 0''	1' - 0''	1' - 6''	1' - 0''	1' - 0''	1' - 0''	3' - 6''	maximum
16'	1' - 0''	1' - 0''	1' - 0''	4' - 6''	1' - 0''	1' - 0''	1' - 0''	7' - 0''	1' - 0''	1' - 0''	1' - 0''	2' - 6"	1' - 0''	1' - 0''	1' - 0''	4' - 6''	40 psf live 15 psf de
18' 20'	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	6' - 0'' 7' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 6'' 2' - 6''	8' - 6'' 9' - 6''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	3' - 6'' 5' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	5' - 6'' 7' - 0''	on simple
22'	1' - 0''	1' - 0''	1' - 6''	8' - 6''	1' - 0''	1' - 0''	3' - 6''	*	1' - 0''	1' - 0''	1' - 0''	6' - 0''	1' - 0''	1' - 0''	2' - 0''	8' - 0''	applicatio
24'	1' - 0''	1' - 0''	2' - 6''	9' - 6''	1' - 0''	1' - 0''	5' - 0''	*	1' - 0''	1' - 0''	1' - 0''	7' - 0''	1' - 0''	1' - 0''	3' - 6''	9' - 6''	3. For other
26' 28'	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	3' - 6'' 4' - 6''	11' - 0'' 12' - 0''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	6' - 0'' 7' - 0''	*	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	2' - 6'' 3' - 6''	8' - 6'' 9' - 6''	1' - 0'' 1' - 0''	1' - 0'' 1' - 0''	4' - 6'' 5' - 6''	10' - 6'' 12' - 0''	condition
30'	1'-0''	1'-0''	4 - 0 5' - 6''	12 - 0	1'-0''	2' - 0''	7 - 0 8' - 6''	*	1' - 0''	1' - 0''	4' - 6''	9 - 0 11' - 0''	1'-0''	1' - 0''	6' - 6''	13' - 0''	sizes, cor
32'	1' - 0''	1' - 0''	7' - 0''	14' - 6''	1' - 0''	3' - 0''	9' - 6''	*	1' - 0''	1' - 0''	5' - 6''	12' - 0''		2' - 6''	8' - 0''	14' - 6''	local distr
34'	1' - 0''	1' - 6''	8' - 0''	16' - 0''	1' - 0''	4' - 6''	11' - 0''	*	1' - 0''	1' - 0''	6' - 6''	13' - 6''	1' - 0''	3' - 6''	9' - 0''	15' - 6''	4. It may be
TABL	_E 5						S	QUA	RE F	IOLE	S						exceed th of those t
Min	imum (distance	≏ from i	nside fa	ace of a	any sun						OIST D	EPTH	• HOLE	SIZE	'INI	analysing
		18				20				2				24			situation
Span [ft]	3''	6"	9''	12"	6"	20 9'') 12''	15''	6"	2. 9''	2 12''	15''	9''	12''	+ 15''	18''	CALC [®] S
8'	1' - 0"	1' - 0"	9 1' - 0"	1' - 0"	1' - 0"	9 1' - 0"	1' - 0"	1' - 6"	1' - 0"	9 1' - 0"	1' - 0"	1' - 0"	9 1' - 0"	1' - 0"	1' - 0"	3' - 0"	5. * = Holes
10'	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 0"	3' - 0"	1' - 0"	1' - 0"	1' - 0"	1' - 6"	1' - 0"	1' - 0"	1' - 0"	4' - 6"	acceptab your loca
12'	1' - 0"	1' - 0"	1' - 0"	1' - 6"	1' - 0"	1' - 0"	1' - 0"	4' - 0"	1' - 0"	1' - 0"	1' - 0"	2' - 6"	1' - 0"	1' - 0"	1' - 6"	5' - 6"	your loca
14' 16'	1' - 0" 1' - 0"	1' - 0" 1' - 0"	1' - 0'' 1' - 0''	3' - 0" 4' - 0"	1' - 0" 1' - 0"	1' - 0" 1' - 0"	1' - 6" 2' - 6"	5' - 6" 6' - 6"	1' - 0" 1' - 0"	1' - 0" 1' - 0"	1' - 0" 1' - 6"	4' - 0" 5' - 0"	1' - 0" 1' - 0"	1' - 0" 1' - 0"	2' - 6" 4' - 0"	*	
18'	1' - 0"	1' - 0"	1' - 6"	4 - 0 5' - 0"	1' - 0"	1' - 0"	2 - 0 3' - 6"	8' - 0"	1' - 0"	1' - 0"	2' - 6"	6' - 6"	1' - 0"	1' - 6"	5' - 0"	*	
20'	1' - 0"	1' - 0"	2' - 6"	6' - 6"	1' - 0"	1' - 6"	5' - 0"	9' - 0"	1' - 0"	1' - 0"	3' - 6"	7' - 6"	1' - 0"	2' - 6"	6' - 0"	*	
22'	1' - 0" 1' - 0"	1' - 0"	3' - 6"	7' - 6" 9' - 0"	1' - 0" 1' - 0"	2' - 6"	6' - 0"	10' - 6" 11' - 6"	1' - 0"	1' - 0"	4' - 6" 6' - 0"	9' - 0"	1' - 0"	3' - 6" 4' - 6"	7' - 6"	*	
24' 26'	1' - 0'' 1' - 0''	1' - 6" 2' - 6"	5' - 0'' 6' - 0''	<u>9' - 0''</u> 10' - 0''	1' - 0" 1' - 0"	3' - 6" 4' - 6"	7' - 0" 8' - 6"	*	<mark>1' - 0"</mark> 1' - 0"	2' - 0" 3' - 0"	6' - 0" 7' - 0"	<mark>10' - 0"</mark> 11' - 6"	1' - 0" 2' - 0"	4' - 6" 5' - 6"	<mark>8' - 6"</mark> 10' - 0"	*	
28'	1' - 0"	3' - 6"	7' - 0"	11' - 6"	2' - 0"	5' - 6"	9' - 6"	*	1' - 0"	4' - 6"	8' - 0"	12' - 6"	3' - 0"	7' - 0"	11' - 0"	*	
30'	1' - 0"	4' - 6"		12' - 6"	3' - 0"	7' - 0"	11' - 0"	*	2' - 0"	5' - 6"	9' - 6"	14' - 0"	4' - 0"	8' - 0"	12' - 6"	*	
32' 34'	2' - 6" 3' - 6"	5' - 6" 7' - 0"	9' - 6"	<mark>14' - 0"</mark> 15' - 0"	4' - 6"	<mark>8' - 0"</mark> 9' - 0"	<mark>12' - 0"</mark> 13' - 6"	*	3' - 0" 4' - 0"	6' - 6" 7' - 6"	10' - 6"	<mark>15' - 0"</mark> 16' - 6"	5' - 6" 6' - 6"	<mark>9' - 0"</mark> 10' - 6"	13' - 6"	*	
		7-0	10 - 0	13 - 0	5-0								0-0	10 - 0	13 - 0		
TABI	LE 6					F	REC	TANG	SULA	RH0	OLES	6					
Min	nimum o	distance	e from i	inside fa	ace of a	any sup	port to	the cer	nterline	of hole	J	OIST D	EPTH	• HOLE	SIZE	[IN]	
Span		18				20	-				2"			2	4"		
	10"x18"			12"x18"	12"x16"			14"x18"	12"x18"	14"x16"	14"x18"	16"x18"	14"x18"		16"x18"	16''x20''	
8'	1' - 6"	1' - 0"	1' - 6"	3' - 0"	1' - 0"	1' - 6"	1' - 6"	3' - 0"	1' - 0"	1' - 0"	2' - 0"	3' - 0"	1' - 0"	2' - 0"	2' - 0"	3' - 6"	
10'	2' - 6"	1' - 6"	3' - 0"	4' - 0"	1' - 6"	3' - 0"	3' - 0"	4' - 6"	2' - 0"	1' - 6"	3' - 0"	4' - 6"	2' - 0"	3' - 6"	3' - 0"	*	
12' 14'	4' - 0" 5' - 0"	3' - 0" 4' - 0"	4' - 0" 5' - 6"	5' - 6" 6' - 6"	2' - 6" 4' - 0"	4' - 0" 5' - 6"	4' - 0" 5' - 6"	5' - 6" *	3' - 0" 4' - 0"	3' - 0" 4' - 0"	4' - 0" 5' - 6"	5' - 6" *	3' - 0" 4' - 0"	4' - 6" 6' - 0"	4' - 6" 5' - 6"	*	
16'	5 - 0 6' - 6"	4 - 0 5' - 0"	5 - 6 6' - 6"	*	4 - 0 5' - 0"	5 - 6 6' - 6"	5 - 6 6' - 6"	*	4 - 0 5' - 6"	4 - 0 5' - 0"	5 - 6 6' - 6"	*	4 - 0 5' - 6"	6 - 0 7' - 0"	5 - 0 7' - 0"	*	
18'	7' - 6"	6' - 6"	8' - 0"	*	6' - 6"	8' - 0"	8' - 0"	*	6' - 6"	6' - 6"	8' - 0"	*	6' - 6"	8' - 6"	8' - 0"	*	
20'	9' - 0"	7' - 6"	9' - 0"	*	7' - 6"	9' - 0"	9' - 0"	*	7' - 6"	7' - 6"	9' - 0"	*	8' - 0"	9' - 6"	9' - 6"	*	
	10' - 0"	9' - 0" 10' - 0"	10' - 6"	*	9' - 0" 10' - 0"	10' - 6" 11' - 6"		*	9' - 0" 10' - 6"	9' - 0" 10' - 0"	10' - 6" *	*	9' - 0" 10' - 6"	*	10' - 6" *	*	
2/1	11-0		11 - 0	*	10 - 0 11' - 6"	*	*	*	10 - 6 11' - 6"		*	*	10 - 6 11' - 6"	*	*	*	
24' 26'	12' - 6"	11' - 6"	*		11 - 0								-				
	12' - 6" *	11' - 6" 12' - 6"	*	*	12' - 6"	*	*	*	13' - 0"	12' - 6"	*	*	13' - 0"	*	*	*	
26'			* * *	*		* * *	* * *	* * *	14' - 0"	12' - 6" 14' - 0" 15' - 6"	* * *	* *	13' - 0" 14' - 6" 15' - 6"	* * *	* * *	* * *	

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- d hole your tor.
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- y be ontact tributor.

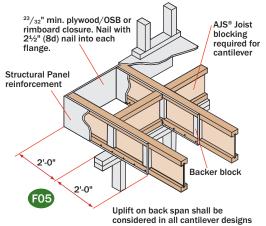
Reinforced Load Bearing Cantilever Detail



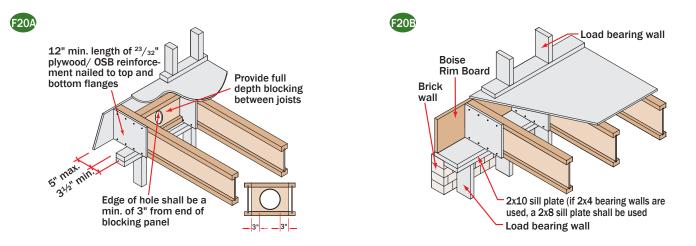
 The tables and details on pages 12 and 13 indicate the type of reinforcements, if any, that are required for load-bearing cantilevers up to a maximum length of 2'-0". Cantilevers longer than 2'-0" cannot be reinforced.
 However, longer cantilevers with lower loads may be allowable without reinforcement. Analyze specific applications with the BC CALC[®] software.

PLYWOOD / OSB REINFORCEMENT (If Required per Table on page 13)

- ²³/₃₂" Min. x 48" long plywood / OSB rated sheathing must match the full depth of the AJS[®] Joist. Nail to the AJS[®] Joist with 2½" (8d) nails at 6" o.c. and nail with 4-2½" (8d) nails into backer block. When reinforcing both sides, stagger nails to limit splitting. Install with horizontal face grain.
- These requirements assume a 100 PLF wall load and apply to AJS[®] Joists. Additional support may be required for other loadings. See BC CALC[®] software.
- Contact Boise Cascade EWP Engineering for reinforcement requirements on AJS[®] Joist depths greater than 16".

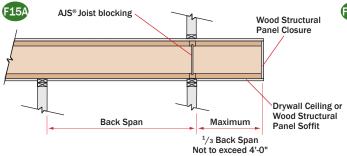


Brick Ledge Load Bearing Cantilever Details



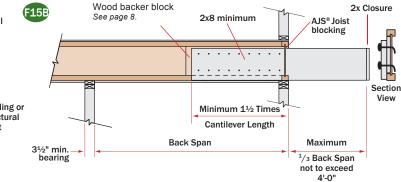
Non-Load Bearing Wall Cantilever Details

AJS[®] Joists are intended only for applications that provide permanent protection from the weather.



Fasten the 2x8 minimum to the AJS[®] Joist by nailing through the backer block and joist web with 2 rows of 3" (10d) nails at 6" on center. Use $3\frac{1}{2}$ " (16d) nails with AJS[®] 25 joists. Clinch all nails.

- These details apply to cantilevers with uniform loads only.
- It may be possible to exceed the limitations of these details by analysing a specific application with the BC CALC[®] software.



Reinforced Load Bearing Cantilever Tables

AJS[®] Joists

		Roof			Sp	ecified	Snow	Load [p	sf]	=0	
Joist Depth	Joist	Truss Span		30		loiot	40 Spacin	a [in]		50	
[in]	Series	[ft]	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	2	0	2	X	1	Χ	X
		26	0	1	X	1	X	X	2	X	X
		28	0	1	X	1	X	X	X	X	X
		30	0	1	X	1	X	X	X	X	X
		32	0	2	X	X	X	X	X	X	X
	AJS® 140	34	1	X	X	X	X	X	X	X	X
	140										
		36	1	Х	Х	X	Х	Х	Х	X	X
		38	1	Х	Х	X	Х	Х	Х	X	X
		40	1	Х	X	X	Х	Х	Х	X	X
		42	2	Х	Х	Х	Х	Х	Х	Х	Х
		24	0	0	2	0	2	Х	1	Х	Х
		26	0	1	Х	1	Х	Х	2	Х	Х
		28	0	1	Х	1	Х	Х	Х	Х	Х
		30	0	1	Х	2	Х	Х	Х	Х	Х
N_∃	AJS®	32	0	2	Х	2	Х	Х	Х	Х	Х
91⁄2"	20	34	1	2	Х	Х	Х	Х	Х	Х	Х
		36	1	Х	Х	Х	Х	Х	Х	Х	Х
		38	1	Х	Х	Х	Х	Х	Х	Х	Х
		40	2	Х	Х	Х	Х	Х	Х	Х	Х
		42	2	Х	Х	Х	Х	Х	Х	Х	Х
		24	0	0	X	0	X	X	2	X	X
		26	0	1	X	1	X	X	2	X	X
		28	0	1	X	2	X	X	X	X	X
		30	0	2	X	2	X	X	X	X	X
	A 10®	30	0					X			
	AJS® 25			2	X	X	X		X	X	X
	25	34	1	Х	Х	X	Х	Х	Х	X	X
		36	1	Х	Х	Х	Х	Х	Х	Х	Х
		38	2	Х	Х	Х	Х	Х	Х	Х	Х
		40	2	Х	Х	Х	Х	Х	Х	Х	Х
		42	Х	Х	Х	Х	Х	Х	Х	Х	Х
		24	0	0	0	0	0	1	0	1	Х
		26	0	0	WS	0	0	Х	0	1	Х
		28	0	0	1	0	1	Х	0	1	Х
		30	0	0	1	0	1	Х	1	Х	Х
	AJS®	32	0	0	1	0	1	Х	1	Х	Х
	140	34	0	0	2	0	2	Х	1	Х	Х
		36	0	0	Х	1	Х	Х	1	Х	Х
		38	0	1	Х	1	Х	Х	Х	Х	Х
		40	0	1	X	1	X	X	X	X	X
		42	0	1	X	1	X	X	X	X	X
		24	0	0	WS	0	0	X	0	1	X
		24	0	0	WS	0	WS	X	0	1	X
		28	0	0	1	0	1	X	0	2	X
					1		1	X	1	X	X
-	A 100	30	0	0		0					
117/8"	AJS®	32	0	0	X	0	1	X	1	X	X
÷	20	34	0	0	Х	0	2	Х	1	X	X
		36	0	WS	X	1	X	X	2	X	X
		38	0	1	Х	1	Х	Х	2	Х	Х
		40	0	1	Х	1	Х	Х	Х	Х	Х
		42	0	1	Х	2	Х	Х	Х	Х	Х
		24	0	0	0	0	0	2	0	1	Х
		26	0	0	0	0	0	Х	0	1	Х
		28	0	0	1	0	1	Х	0	2	Х
		30	0	0	1	0	1	Х	1	Х	Х
	AJS®	32	0	0	2	0	2	Х	1	X	Х
	25	34	0	0	2	0	2	X	2	X	X
		36	0	0	X	1	X	X	2	X	X
		38	0	1	X	1	X	X	X	X	X
		40	0	1	X	2	X	X	X	X	X
		40	0	1	X	2	X	X	X	X	X
		42	0		~	2	~	~	~	~	~

KEY TO TABLE

- 0 No Reinforcement Required
- WS Web Stiffeners at Support
- Web Stiffeners Plus One Reinforcer
 Web Stiffeners Plus Two Reinforcers
 Web Stiffeners Plus Two Reinforcers
 Use Deeper Joists or Closer Spacing

Tables are based on the following loads: 15 psf specified floor dead load, 40 psf specified floor live load, 100 plf specified wall dead load, 10 psf specified roof dead load and the listed specified snow load (Standard Term Load Duration).

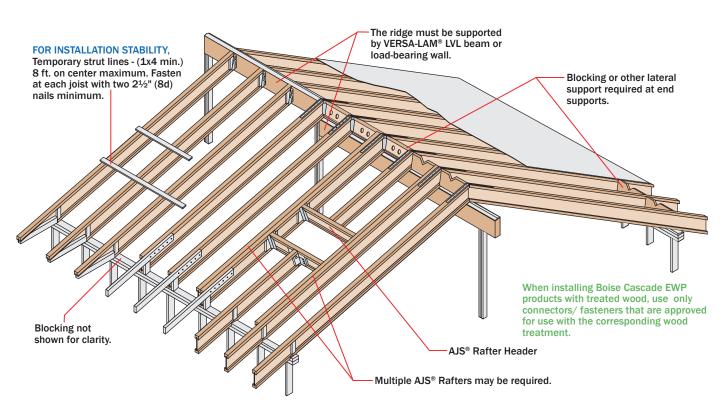
-		Roof			Sn	ocified	Snow	l oad [r	oofl			
Jois	st	Truss		30	Sp	ecilieu	40		51]	50		
Dep	th Joist	Span		00		Joist	Spacir	na [in]	1	00		
[in]] Series	; [ft]	16	19.2	24	16	19.2	24	16	19.2	24	
		24	0	0	WS	0	0	WS	0	WS	1	
		26	0	0	WS	0	WS	1	0	WS	Х	
		28	0	0	WS	0	WS	1	0	WS	X	
		30	0	0	WS	0	WS	X	WS	1	Х	
	AJS	32	0	0	WS	0	WS	Х	WS	1	Х	
	20	34	0	WS	WS	0	WS	Х	WS	1	Х	
		36	0	WS	1	WS	1	Х	WS	Х	Х	
		38	0	WS	1	WS	1	Х	1	Х	Х	
		40	0	WS	1	WS	1	Х	1	Х	Х	
-		42	0	WS	Х	WS	Х	Х	1	Х	Х	
4		24	0	0	0	0	0	WS	0	0	1	
		26	0	0	0	0	0	1	0	0	2	
				-			-		-	-		
		28	0	0	0	0	0	1	0	WS	2	
		30	0	0	0	0	0	2	0	1	Х	
	AJS	32	0	0	WS	0	0	2	0	1	Х	
	25	34	0	0	WS	0	WS	Х	0	1	Х	
		36	0	0	1	0	1	Х	0	2	Х	
		38	0	0	1	0	1	Х	1	2	Х	
		40	0	0	1	0	1	Х	1	Х	Х	
		42	0	0	2	0	2	X	1	X	X	
		24	0	0	_ WS	0	0	WS	0	WS	WS	
		24	0	0	WS	0	WS	WS	0	WS	WS	
				-			-		-			
		28	0	0	WS	0	WS	WS	0	WS	1	
		30	0	0	WS	0	WS	WS	WS	WS	Х	
	AJS®	32	0	0	WS	0	WS	1	WS	WS	Х	
	20	34	0	WS	WS	WS	WS	1	WS	WS	Х	
		36	0	WS	WS	WS	WS	Х	WS	WS	Х	
		38	0	WS	WS	WS	WS	Х	WS	1	Х	
		40	0	WS	WS	WS	WS	Х	WS	1	Х	
		42	0	WS	WS	WS	WS	X	WS	X	X	
16"	2	24	0	0	0	0	0	0	0	0	WS	
								-		-		
		26	0	0	0	0	0	WS	0	0	WS	
		28	0	0	0	0	0	WS	0	0	1	
		30	0	0	0	0	0	WS	0	WS	1	
	AJS [∉]	32	0	0	0	0	0	1	0	WS	2	
	25	34	0	0	WS	0	WS	1	0	WS	2	
		36	0	0	WS	0	WS	1	0	WS	Х	
		38	0	0	WS	0	WS	2	WS	1	Х	
		40	0	0	WS	0	WS	2	WS	1	X	
		40	0	0	WS	0	WS	X	WS	2	X	
		-										
		24	WS									
		26	WS									
		28	WS									
Ę.,		30	WS									
18" to 24'	AJS	32	WS									
= to	25	34	WS									
9	2	36	WS									
		38	WS									
		40	WS WS									
		42										

NOTES:

- Cut 48" long reinforcers to match the joist depth. Use min. 23/32" APA Rated 1 Sheathing, Exposure 1, 48/24 Span Rating panels. The face grain must be horizontal (measure the 48" dimension along the long edge of the panel).
- 2. Minimum bearing length is 31/2".
- Fasten the reinforcer to the joist flanges with 21/2" (8d) nails at 6" o.c. When 3. reinforcing both sides, stagger the nails to limit splitting the joist flanges.
- Attach web stiffeners per intermediate Web Stiffener Nailing Schedule on 4. page 9.
- Use the BC CALC® software to analyze conditions that are not covered by 5. this table. It may be possible to exceed the limitations of this table by analyzing a specific application with BC CALC® software.

Roof Framing

AJS[®] Rafters



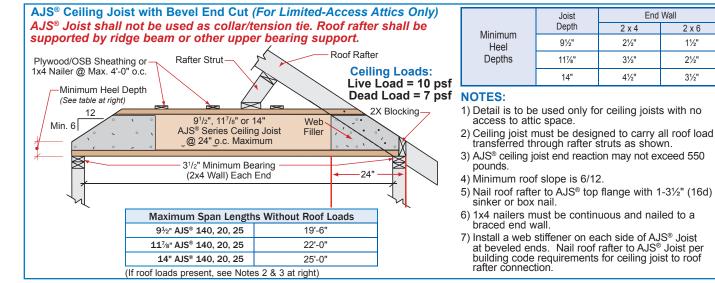
SAFETY WARNING

DO NOT ALLOW WORKERS ON AJS® JOISTS UNTIL ALL HANGERS, AJS® RIM JOISTS, RIM BOARDS, AJS® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of AJS® Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of AJS® Joists at the end of the bay.
- All hangers, AJS® rim joists, rim boards, AJS® blocking panels, and x-bracing must be completely installed and properly nailed as each AJS® Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional AJS[®] Joists are set. Nail the strut lines to the sheathed area. or braced end wall, and to each AJS® Joist with two 21/2" (8d) nails.

SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the AJS $^{\circ}$ Joist to within $\frac{1}{2}$ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.



2 x 6

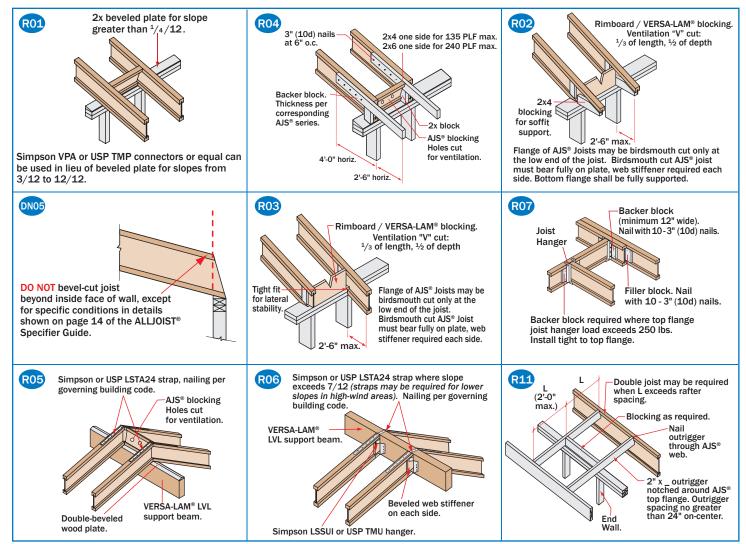
11/2"

21/2"

31/2"

Roof Framing Details

Additional roof framing details available with BC FRAMER[®] software



LATERAL SUPPORT

- AJS[®] Joists must be laterally supported at the ends with hangers, AJS[®] rim joists, rim boards, AJS[®] blocking panels or x-bracing. AJS[®] blocking panels or x-bracing are required at cantilever supports.
- Blocking may be required at intermediate bearings for floor diaphragm as per Code, consult local building official.

MINIMUM BEARING LENGTH FOR AJS® JOISTS

- 1% inches is required at end supports (1% inches for 18" to 24" deep). 3% inches is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC CALC® software.

NAILING REQUIREMENTS

- AJS^{\circledast} rim joist, rim board or closure panel to AJS^{\circledast} Joist:
- Rims or closure panel 1³/₄ inches thick and less 2-21/2" (8d) nails, one each in the top and
- bottom flange.
- AJS[®] 140 / 20 rim joist: 2-3¹/₂" (16d) box nails, one each in the top and bottom flange
- AJS[®] 25 rim joist: Toe-nail top flange to rim joist with 2-3" (10d) box nails, one each side of flange.
- AJS® rim joist, rim board or AJS® blocking panel to support:
- 21/2" (8d) nails at 6 inches on center. ALLJOIST® Specifier Guide - CANADA

- When used for shear transfer, follow the building designer's specification.
- · AJS® Joist to support:
- AJS® Joist to Support:
 2-2½" (8d) nails, one on each side of the web, placed 1½ inches minimum from the end of the AJS® Joist to limit splitting.
 Sheathing to AJS® Joist:
 Prescriptive residential roof sheathing nailing requires 2½" (8d) common nails @ 6" o.c. on edges and @ 12" o.c. in the field as per Code
- as per Code.
- Maximum nail spacing for minimum lateral stability = 24"
- 14 gauge staples may be substituted for $2\frac{1}{2}$ " (8d) nails if the staples point to the staples point of the s least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.

BACKER AND FILLER BLOCK DIMENSIONS

Series	Backer Block Thickness	Filler Block Thick- ness
AJS® 140	1 ¹ / ₈ " or two ¹ / ₂ " wood panels	2 x + 5%" wood panel
AJS [®] 20	1 ¹ / ₈ " or two ¹ / ₂ " wood panels	2 x + 5%" wood panel
AJS [®] 25	2 x _ lumber	Double 2 x lumber

- . Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced fit.
- For deeper AJS® 25 Joists, stack 2x lumber or use multiple pieces of 3/4" wood panels.

WEB STIFFENER REQUIREMENTS

See Web Stiffener Requirements on page 9.

MAXIMUM SLOPE

Unless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.

VENTILATION

 The 1½ inch, pre-stamped knock-out holes spaced at 12 inches on center along the AJS[®] Joist may all be knocked out and used for cross ventilation. Deeper joists than what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specific ventilation requirements.

BIRDSMOUTH CUTS

 AJS^{\circledast} Joists may be birdsmouth cut only at the low end support. AJS^{\circledast} Joists with birdsmouth cuts may cantilever up to 2'-6" past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut.

PROTECT AJS® JOISTS FROM THE WEATHER

AJS® Joists are intended only for applications that provide permanent protection from the weather. Bundles of AJS[®] Joists should be covered and stored off of the ground on stickers.

Roof Span Tables

Loodo	Cariaa	Depth	Lo	w Roof Slo	ре	Hi	gh Roof Slo	ре
Loads	Series	(in)	16"	19.2"	24"	16"	19.2"	24"
	AJS®	91⁄2"	19'-8"	18'-9"	17'-8"	17'-4"	16'-6"	15'-7"
	140	11%"	22'-5"	21'-5"	20'-3"	19'-10"	18'-11"	17'-10"
		91⁄2"	20'-10"	19'-11"	18'-9"	18'-5"	17'-7"	16'-7"
psf) psf	AJS®	111/8"	23'-9"	22'-8"	21'-5"	21'-0"	20'-0"	18'-11"
10 р 20 р	20	14"	26'-2"	25'-0"	23'-7"	23'-1"	22'-1"	20'-10"
н п		16"	28'-3"	27'-0"	25'-6"	25'-0"	23'-10"	22'-6"
Loa / Loa		91⁄2"	22'-7"	21'-7"	20'-4"	19'-11"	19'-0"	18'-0"
Dead Snow		117⁄8"	25'-9"	24'-7"	23'-2"	22'-9"	21'-8"	20'-6"
ied [ied \$		14"	28'-4"	27'-0"	25'-6"	25'-0"	23'-10"	22'-6"
Specified Dead Load = Specified Snow Load =	AJS®	16"	30'-7"	29'-2"	27'-6"	27'-0"	25'-9"	24'-4"
S S	25	18"	32'-10"	31'-4"	29'-7"	29'-0"	27'-8"	26'-1"
		20"	34'-10"	33'-3"	31'-5"	30'-9"	29'-4"	27'-9"
		20"	36'-9"	35'-1"	33'-2"	32'-6"	31'-0"	29'-3"
		24"	38'-7"	36'-10"	34'-10"	34'-1"	32'-7"	30'-9"
	AJS®	91⁄2"	18'-3"	17'-5"	16'-6"	16'-3"	15'-6"	14'-8"
	140	111/8"	20'-11"	19'-11"	18'-10"	18'-7"	17'-8"	16'-9"
		91⁄2"	19'-5"	18'-6"	17'-6"	17'-3"	16'-5"	15'-6"
psf psf	AJS®	111/8"	22'-2"	21'-2"	19'-11"	19'-8"	18'-9"	17'-9"
10 р 30 р	20	14"	24'-5"	23'-3"	22'-0"	21'-8"	20'-8"	19'-6"
Load = Load =		16"	26'-4"	25'-2"	23'-9"	23'-5"	22'-4"	21'-1"
		91⁄2"	21'-0"	20'-1"	18'-11"	18'-8"	17'-10"	16'-10"
Dead Snow		117⁄8"	24'-0"	22'-11"	21'-7"	21'-4"	20'-4"	19'-2"
ied D		14"	26'-4"	25'-2"	23'-9"	23'-5"	22'-4"	21'-1"
Specified Dead Specified Snow	AJS®	16"	28'-5"	27'-2"	25'-7"	25'-3"	24'-1"	22'-9"
0 0	25	18"	30'-7"	29'-2"	27'-6"	27'-2"	25'-11"	24'-6"
		20"	32'-5"	31'-0"	29'-3"	28'-10"	27'-6"	26'-0"
		22"	34'-3"	32'-8"	30'-10"	30'-5"	29'-0"	27'-5"
		24"	36'-0"	34'-4"	32'-5"	31'-11"	30'-6"	28'-9"

NOTES:

- Spans apply to simple span application only.
- For $9\frac{1}{2}$ " to 16" depth, the minimum end bearing lengths are $1\frac{1}{2}$ " at high end and $3\frac{1}{2}$ " at lower end.
- For 18" and deeper depth, the minimum end bearing lengths are 1³/₄" at high end and 3¹/₂" at lower end.
- Stiffeners required at ALL bearing locations for all 18" to 24" deep joists (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured between the supports (clear span) of bearing and are based on uniformly loaded joists.
- Live load deflection is limited to L/240 or 2". Total load deflection is limited to L/180 or 1".
- Refer to appropriate sections of the AllJoist Specifier Guide for installation guidelines and construction details.
- Allowable spans assume no composite action provided by sheathing.
- Low roof slope is from $\frac{1}{4}$ /12 to 6/12.
- High roof slope is from 6/12 to 12/12.
- Table assumes a 2 foot roof overhang.
- It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC[®] software.
- Slope roof joists at least ¼" over 12" to minimize ponding.
- Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.10. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the roof. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

Roof Span Tables

cified Dead Load = 10 psf cified Snow Load = 40 psf	AJS [®] 140 AJS [®] 20	Depth (in) 9½" 11%" 9½" 11%" 11%" 11%" 116"	16" 17'-4" 19'-9" 18'-4" 20'-11"	19.2" 16'-6" 18'-10" 17'-6" 20'-0"	24" 14'-7" 16'-8" 16'-6"	16" 15'-5" 17'-7" 16'-4"	19.2" 14'-8" 16'-10"	24" 13'-11" 15'-10"
= 10 psf = 40 psf	140 AJS®	117⁄8" 91⁄2" 117⁄8" 14"	19'-9" 18'-4" 20'-11"	18'-10" 17'-6"	16'-8"	17'-7"	16'-10"	
11 11	AJS®	9½" 11½" 14"	18'-4" 20'-11"	17'-6"				15'-10"
11 11		117⁄₅" 14"	20'-11"		16'-6"	16'-4"	451.7"	
11 11		14"		20'-0"			15'-7"	14'-9"
11 11	20				18'-10"	18'-8"	17'-10"	16'-10"
11 11		16"	23'-1"	22'-0"	20'-3"	20'-7"	19'-7"	18'-6"
Specified Dead Loa Specified Snow Loa			24'-11"	23'-9"	20'-5"	22'-2"	21'-2"	19'-6"
Specified Dead Specified Snow		9½"	19'-11"	19'-0"	17'-11"	17'-9"	16'-11"	16'-0"
Specified [Specified 9		117⁄8"	22'-8"	21'-8"	20'-2"	20'-2"	19'-3"	18'-2"
Specif		14"	24'-11"	23'-9"	20'-3"	22'-3"	21'-2"	19'-3"
<u>0</u> 0	AJS®	16"	26'-11"	25'-8"	20'-5"	24'-0"	22'-11"	19'-6"
	25	18"	28'-11"	27'-7"	26'-0"	25'-9"	24'-7"	23'-3"
		20"	30'-8"	29'-3"	27'-8"	27'-4"	26'-1"	24'-8"
		22"	32'-5"	30'-11"	29'-2"	28'-10"	27'-6"	26'-0"
		24"	34'-0"	32'-5"	30'-7"	30'-4"	28'-11"	27'-4"
	AJS®	91⁄2"	16'-6"	14'-11"	13'-4"	14'-9"	14'-1"	13'-0"
	140	117⁄8"	18'-11"	17'-0"	15'-2"	16'-10"	16'-1"	14'-11"
		9½"	17'-6"	16'-9"	15'-9"	15'-8"	14'-11"	14'-1"
psf	AJS®	117⁄8"	20'-0"	19'-1"	16'-8"	17'-10"	17'-1"	16'-1"
10 psf 50 psf	20	14"	22'-0"	21'-0"	16'-9"	19'-8"	18'-9"	16'-1"
		16"	23'-9"	21'-3"	16'-11"	21'-3"	20'-3"	16'-3"
		9½"	19'-0"	18'-1"	16'-7"	17'-0"	16'-2"	15'-3"
Dead		117⁄8"	21'-8"	20'-8"	16'-8"	19'-4"	18'-5"	16'-0"
ied D		14"	23'-10"	21'-0"	16'-9"	21'-3"	20'-4"	16'-1"
Specified Dead Specified Snow	AJS®	16"	25'-8"	21'-3"	16'-11"	22'-11"	20'-5"	16'-3"
N N	25	18"	27'-7"	26'-4"	24'-10"	24'-8"	23'-6"	22'-3"
		20"	29'-4"	27'-11"	26'-5"	26'-2"	25'-0"	23'-7"
		22"	30'-11"	29'-6"	27'-10"	27'-8"	26'-4"	24'-11"

NOTES:

- Spans apply to simple span application only.
- For 9¹/₂" to 16" depth, the minimum end bearing lengths are 1¹/₂" at high end and 3¹/₂" at lower end.
- For 18" and deeper depth, the minimum end bearing lengths are 1³/₄" at high end and 3¹/₂" at lower end.
- Stiffeners required at ALL bearing locations for all 18" to 24" deep joists (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured between the supports (clear span) of bearing and are based on uniformly loaded joists.
- Live load deflection is limited to L/240 or 2". Total load deflection is limited to L/180 or 1".
- Refer to appropriate sections of the AllJoist Specifier Guide for installation guidelines and construction details.
- Allowable spans assume no composite action provided by sheathing.
- Low roof slope is from $\frac{1}{4}$ /12 to 6/12.
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- Table assumes a 2 foot roof overhang.
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load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the roof. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

Limit States Design - Standard Term Load Duration Deflection Criteria: L/240 (Live Load) & L/180 (Total Load)

AJS® 140 Series

		21/2" Flange Width												
		91⁄2" AJS® 140			11 7∕₃" AJS® 140									
	Deflection (unfactored)		Deflection (
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance								
8			366			376								
10			292			301								
12			227			301								
14			167			215								
16			127			165								
18	65		101			130								
20	48		81			106								
22	36		67			87								
24	28	37	56	47		73								
26	22	29	48	37		62								

	PSF to PLF CONVERSION TABLE										
Joist		LOAD (psf)									
Spacing	20	25	30	35	40	45	50	60			
12"	20	25	30	35	40	45	50	60			
16"	27	33	40	47	53	60	67	80			
19.2"	32	40	48	56	64	72	80	96			
24"	40	50	60	70	80	90	100	120			

TO CONVERT FROM SPECIFIED LOAD (PLF) TO FACTORED LOAD (PLF) - Factored (PLF) = 1.25 x Specified Dead Load (PLF) + 1.50 x Specified Live/Snow Load (PLF)

GENERAL NOTES:

- 1. The spans listed are the clear span distance between supports.
- Table is valid for simple and continuous span applications under uniform load.
- 3. Design of continuous spans shall be based on the longest span. The shorter span shall not be less than 50% of the longest length.
- 4. Table assumes a fully laterally restrained compression flange.
- 5. The repetitive member factor, K_{H} is not applicable.
- 6. Table assumes a minimum bearing length with no stiffeners.

	DEAD LOAD SLOPE FACTOR										
Joist Pitch	2/12	3/12	4/12	5/12	6/12	7/12	8/12	9/12	10/12	11/12	12/12
Slope Factor	1.014	1.031	1.054	1.083	1.118	1.158	1.202	1.250	1.302	1.357	1.414

How to calculate a roof plf loading:

Joist: Spacing: Span: Roof Pitch:	16'-0"	
Snow Load :	SL = 30 lbs/ft ²	
Dead Load :	DL = 10 lbs/ft ²	
Design Criteria Live Load Defle Total Load Defle	ection: L/240	

Applied Load:

 $WSL = SL \times tributary width$ $WSL = \frac{30 \text{ lbs}}{\text{ft}^2} \times \frac{24 \text{ in}}{12 \text{ in / ft}}$ WSL = 60 plf (lbs/ft)

WDL = DL x tributary width x slope factor

 $WDL = \frac{10 \text{ lbs}}{\text{ft}^2} \times \frac{24 \text{ in}}{12 \text{ in / ft}} \times 1.054$ WDL = 21.1 plf (lbs/ft)

WSL = WSL = 60 plf WTL = WSL + WDL = 81.1 plf WF = 1.25 x WDL + 1.50 x WSL = 117 plf (Factored)

Check Capacities :

Live Load (L/240):	60 plf < 115 plf -> ok
Total Load (L/180):	81.1 plf <> ok
Factored Resistance:	117 plf < 177 plf -> ok
Note:	

For roof pitches greater than a 2/12, approximate the increased dead load by multiplying the specified dead load by the slope factor.

Definitions:

WSL	Uniform Snow Load	[lb/ft]
WDL	Uniform Dead Load	[lb/ft]
WTL	Uniform Total Load	[lb/ft]
WF	Uniform Factored Load	[lb/ft]

Limit States Design - Standard Term Load Duration Deflection Criteria: L/240 (Live Load) & L/180 (Total Load)

					AJS	[®] 20 S	eries						
					21/	∕₂" Flan	ge Wic	lth					
	91	∕₂" AJS® 2		11	7∕8" AJS®	20	14	4" AJS® 2	20	1(6" AJS® 2	° 20	
Span		ection ctored)	Factored Strength Resistance	Defle (unfac	ction tored)	Factored Strength Resistance		ection tored)	Factored Strength Resistance		ection ctored)	Factored Strength Resistance	
Length [ft]	L/240 (Live)	L/180 (Total)	Fac Stre Resi	L/240 (Live)	L/180 (Total)	Fac Stre Resi	L/240 (Live)	L/180 (Total)	Fac Stre Resi	L/240 (Live)	L/180 (Total)	Fac Stre Resi	
8			366			376			378			382	
10			292			301			303			306	
12			292			301			303			306	
14			209			215			216			218	
16	115		177			188			189			191	
18	82		140			167			168			170	
20	60		113			147			151			153	
22	46	61	93	77		121			137			139	
24	35	47	78	60		102			122			127	
26	28	37	67	47		86	69		104			117	

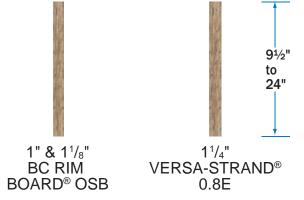
AJS[®] 25 Series

					31/	∕₂" Flan	ge Wid	lth				
	Defle (unfac	ection tored)	ored ngth ance	Defle (unfac		ored ngth ance	Defle (unfac	ection stored)	ored ngth ance	Defle (unfac		ored ngth ance
Span Length	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance
[ft]	91	2" AJS® 2	25	11	‰" AJS®	25	14	4" AJS® 2	25	16	5" AJS® 2	25
8			366			376			378			382
10			292			301			303			306
12			292			250			252			255
14			209			215			216			218
16			183			188			189			191
18			162			167			168			170
20	82		146			150			151			153
22	62	83	133			136			137			139
24	49	65	122	81		125			126			127
26	38	51	105	64	29	115			116			117
	18	8" AJS® 2	25	20)" AJS® 2	25	22	2" AJS® 2	25	24	4" AJS® 2	25
20			298			322			330			337
22			270			293			300			306
24			248			268			275			281
26			216			241			254			259
28			186			208			228			241
30			162			181			198			216
32	91		142			159			174			190
34	76		126			141			154			168
36	64		112	81		125			138			150
38	55		101	70		113			123			134

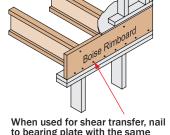
ALLJOIST® Specifier Guide - CANADA

Boise Cascade Rimboard

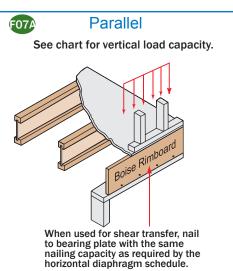
Boise Cascade Rimboard Product Profiles

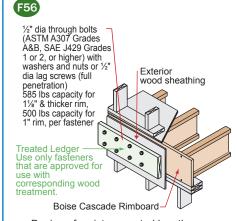






to bearing plate with the same nailing capacity as required by the horizontal diaphragm schedule.





Design of moisture control by others (only structural components shown above)

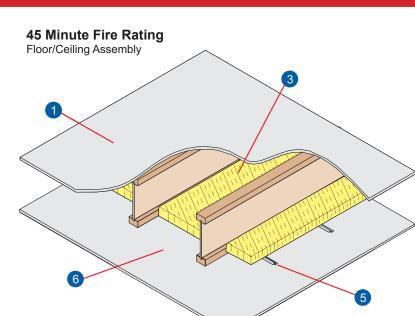
Boise Cascade Rimboard Properties

Pim Poord Type	Thickness	$\phi H^{(1)}$	φV [II	0/ft] ⁽²⁾	φZ ⁽³⁾	φP ⁽⁴⁾
Rim Board Type	[in]	[lb/ft]	d ≤ 16"	d > 16"	[lb]	[lb]
Boise Cascade Rimboard	1"	235	5500	2750	495	5840
Boise Cascade Rimboard	11⁄8"	235	7340	5000	585	5840
Boise Cascade Rimboard Plus	11⁄8"	260	8090	5340	585	5840
Boise Cascade VERSA-STRAND® 0.8E	1¼"	310	9460	5820	830	8990

NOTES:

- φH = Factored horizontal (shear) load transfer capacity is based on the minimum nailing attachment schedule specified in NBCC 2005 and APA document D340CA.
- 2. ϕV = Factored uniform bearing (vertical) load resistance. The uniform bearing load shall be simultaneously satisfied with the concentrated vertical load resistance, when applicable.
- 3. φ Z = Factored lateral resistance of a ½ inch (12.7 mm) diameter lag screw.
- φP = Factored concentrated vertical load resistance based on 4½ inch (114 mm) bearing length. The concentrated vertical load shall be simultaneously satisfied with the uniform bearing load capacity, when applicable.
- 5. All tabulated values are applicable to the standard-term load duration and permitted to be adjusted for other load durations in accordance with CSA 086.
- 6. See CCMC Evaluation Report No. 13143-R for further product information on Boise Cascade VERSA-STRAND 0.8E.

Fire Resistance



3 Insulation

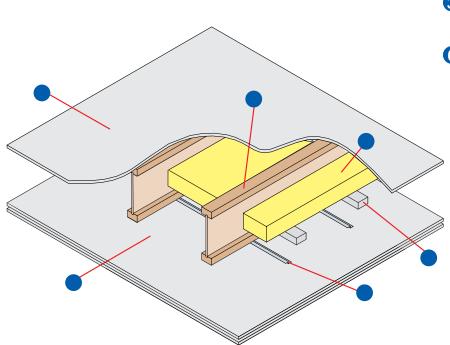
3¹/₂" thick, minimum 2.5 pcf, mineral wool insulation batts.



One layer of ⁵/₈" Type C gypsum wallboard installed perpendicular to channels with end joints staggered 48". Boards to be fastened to channels with minimum 11/₈" Type S drywall screws located 12" on center. Gypsum wallboard joints shall be covered with tape and coated with gypsum joint compound.

REFERENCE:

NBCC 2005, table A 9.10.3.1B. Assembly F5g



3 Insulation (optional) 3¹/₂" fiberglass batt insulation.

6 Gypsum Wallboard

Two layers of %" Type C gypsum wallboard. Base layer installed perpendicular to joists or channels and fastened with 1%" screws located at 12" on center. Face layer installed parallel to base layer with end and edge joints staggered 16" minimum and fastened with 2½" screws located at 12" on center on intermediate joists, and 8" on center at end joints. Gypsum wallboard joints shall be covered with tape and coated with gypsum joint compound.

REFERENCE:

PFS AJS®, Assembly 1

1 Subfloor

Minimum %" plywood or OSB sheathing fastened to joists in accordance with Code specifications. Construction adhesive is optional.

2 Structural Members

 AJS° Joists having a minimum depth of 9½" and spaced at 24" o.c. maximum.

4 Insulation Supports

Nominal 2x3 strapping located 16" o.c. or equivalent method to retain insulation above joist flanges.

5 Resilient Channels

Minimum 25 gauge $\frac{1}{2}$ " offset RC-1 galvanized steel channels installed perpendicular to joists spaced at 16" o.c. maximum and fastened with $\frac{1}{2}$ " screws at each joist intersection.

SINGLE I-JOISTS – Canadian/Factored Resistance (lbs)

Face Mo Uplift Down Load Uplift Down Load Uplift Down Load в Fastener Type Fastener Type Fastener Type Joist в R Model Model Model leigh Dim (115) Dim Header Joist (115)Header Joist DE SPE DE SPE Dim Header Joist (115) DF SPF AJS 140, 20 loist Width 91⁄2 LT259 6-10d 1-#8x1¼ws 2560 1725 IUS2.56/9.5 2 8-10d 105 2385 1700 LF259 2 10-10d 1-#8x1¼ws 100 2525 2155 2 100 2560 1725 IUS2.56/11.88 10-10d 2565 1835 2 2880 2270 111/8 LT251188 2 6-10d 1-#8x1¼ws 100 2 105 LF2511 12-10d 1-#8x1¼ws 100 1-#8x1¼ws LF2514 14 LT2514 2 6-10d 100 2560 1725 IUS2.56/14 2 12-10d 105 2565 1835 2 14-10d 1-#8x1¼ws 100 3235 2385 16 LT2516 2 6-10d -#8x1¼ws 100 2560 1725 IUS2.56/16 2 14-10d 105 2725 1950 MIU2.56/16 21/2 24-16d 2-10dx1½ 270 4930 3485 AJS Joist Width = 3 10-10d 2-#8x1¼ws 91/2 LT359 2 6-10d 2-#8x1¼ws 100 2560 1725 IUS3.56/9.5 2 10-10d 105 2375 1695 LF359 2 100 2525 2155 LT351188 2 2-#8x1¼ws IUS3.56/11.88 2 2375 12-10d 2-#8x1¼ws 2880 2270 111/8 6-10d 100 105 1695 LF3511 2 100 2560 1725 12-10d LT3514 6-10d 2-#8x1¼ws 100 2560 1725 IUS3.56/14 12-10d 105 2375 1695 LF3514 2 14-10d 2-#8x1¼ws 100 3235 2385 14 2 2 LT3516 2 6-10d 2-#8x1¼ws 100 2560 1725 14-10d 105 2375 MIU3.56/16 21⁄2 24-16d 2-10dx1½ 270 4930 3485 16 IUS3.56/16 2 1695 18 MIT418 21/2 8-16d 2-10dx11/2 380 3480 2415 No IUS Hanger for these sizes MIU3.56/18 21/2 26-16d 2-10dx11/2 270 4930 3485 eld Slo Uplift Down Load Uplift Down Load Fastener Type Uplift Down Load В Fastener Type В Fastener Type в Model Model Model leiah (115) (115) DF SPF (115) DF SPF Dim Dim DF SPF Dim Header Joist Header Joist Header Joist AJS 140, 20 Joist Width = 21/2 SUR/L2.56/9 14-16d 2-10dx11/2 3945 2780 THAI322 6-10d 2-10dx11/2 2740 2075 LSSUH310 14-16d 12-10dx1½ 1220 2620 1850 91/2 3¾16 385 21⁄4 31⁄2 11% SUR/L2.56/11 33/16 16-16d 2-10dx11/2 385 3945 2780 THAI322 21/4 6-10d 2-10dx11/2 2740 2075 LSSUH310 31/2 14-16d 12-10dx11/2 1220 2620 1850 6-10d 2-10dx1½ 2740 2075 14-16d 12-10dx1¹/₂ 1220 2620 1850 14 SUR/L2.56/14 33/16 18-16d 2-10dx11/2 385 3945 2780 THAI322 21⁄4 LSSUH310 31/2 See Canadian Wood Construction Connectors Catalogue See Canadian Wood Construction Connectors Catalogue 16 SUR/L2.56/14 33/16 18-16d 2-10dx1½ 385 3945 2780 for hanger selection. for hanger selection. AIS Joist Width : 91/2 SUR/L410 25⁄8 14-16d 6-16d 1395 4065 2875 THAI422 2¼ 6-10d 2-10dx11/2 2740 2075 LSSU410 31⁄2 14-16d 12-10dx11/2 1220 3055 2160 SUR/L410 LSSU410 111% 25/8 14-16d 6-16d 1395 4065 2875 THAI422 21/4 6-10d 2-10dx11/2 2740 2075 31/2 14-16d 12-10dx11/2 1220 3055 2160 14 SUR/L414 21/2 18-16d 8-16d 1555 4095 2895 THAI422 21⁄4 6-10d 2-10dx1½ 2740 2075 LSSU410 31/2 14-16d 12-10dx1½ 1220 3055 2160 16 SUR/L414 21/2 18-16d 8-16d 1555 4095 2895 See Canadian Wood Construction Connectors Catalogue See Canadian Wood Construction Connectors Catalogue SUR/L414 for hanger selection. for hanger selection.

21/2 Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for nonshaded hangers by others.

18-16d

8-16d

1555 4095 2895

4.

18

3. The B Dim is the depth of the hanger seat.

For more information, call Simpson Strong-Tie at 800-999-5099 or visit their website at www.strongtie.com

SIMPSON

Strong-Tie

2. THAI hangers require a minimum of 4 top and 2 face nails installed

DOUBLE I-JOISTS - Canadian/Factored Resistance (lbs)

WS = wood screw

			Тор	Flange						Fac	e Mount						45°	Skew			
Joist	Model	В	Faster	ner Type	Uplift	Down	Load	Model	В	Faster	er Type	Uplift	Down	Load	Model	В	Faster	er Type	Uplift	Dowi	n Load
Height	Model	Dim	Header	Joist	(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF	Model	Dim	Header	Joist	(115)	DF	SPF
Doub	le AJS 140, 2	20								Joist Wie	lth = 5"										
91⁄2	MIT39.5-2	21⁄2	8-16d	2-10dx1½	380	3480	2415	MIU5.12/9	21⁄2	16-16d	2-10dx1½	270	4550	3215	HSUR/L5.12/9	213/16	12-16d	2-10dx1½	195	2995	2350
117⁄8	MIT311.88-2	21⁄2	8-16d	2-10dx1½	380	3480	2415	MIU5.12/11	21⁄2	20-16d	2-10dx1½	270	4550	3215	HSUR/L5.12/11	213/16	16-16d	2-10dx1½	195	4195	2965
14	MIT314-2	21⁄2	8-16d	2-10dx1½	380	3480	2415	MIU5.12/14	21/2	22-16d	2-10dx1½	270	4930	3485	HSUR/L5.12/14	213/16	20-16d	2-10dx1½	195	4195	2965
16	MIT5.12/16	21⁄2	8-16d	2-10dx1½	380	3480	2415	MIU5.12/16	21⁄2	24-16d	2-10dx1½	270	4930	3485	HSUR/L5.12/16	213/16	24-16d	2-10dx1½	195	4195	2965
Doub	le AJS 25								J	oist Wid	th = 7"										
91⁄2	B7.12/9.5	21⁄2	14-16d	6-16d	1170	5940	3910	HU410-2	21⁄2	18-16d	8-16d	1865	5780	4670	HU410-2X ⁴	21/2	18-16d	8-16d	1400	3755	3035
117/8	B7.12/11.88	21/2	14-16d	6-16d	1170	5940	3910	HU412-2	21/2	22-16d	8-16d	1865	5780	4670	HU412-2X ⁴	21/2	22-16d	8-16d	1400	3755	3035
14	B7.12/14	21⁄2	14-16d	6-16d	1170	5940	3910	HU414-2	21⁄2	26-16d	12-16d	2685	7025	5780	HU414-2X ⁴	21/2	26-16d	12-16d	2015	4565	3755
16	B7.12/16	21⁄2	14-16d	6-16d	1170	5940	3910	HU414-2	21⁄2	26-16d	12-16d	2685	7025	5780	HU414-2X ⁴	21/2	26-16d	12-16d	2015	4565	3755
18	B7.12/18	21⁄2	14-16d	6-16d	1170	5940	3910	HU414-2	21⁄2	26-16d	12-16d	2685	7025	5780	HU414-2X ⁴	21⁄2	26-16d	12-16d	2015	4565	3755

		A	djustab	le Height						Field S	lope & Ske	w		
Joist	Model	В	Fasten	er Type	Uplift	Dowi	n Load	Model	В	Faster	ner Type	Uplift	Dowr	1 Load
Height	Model	Dim	Header	Joist	(115)	DF	SPF	woder	Dim	Header	Joist	(115)	DF	SPF
Double	e AJS 14(), 20				Joist	Width	= 5"						
9½	THAI-2 ²	21⁄2	6-10d	2-10dx1½	-	2935	2935	LSU5.12 ³	3½	24-16d	16-10dx1½	910	2600	1835
11%	THAI-2 ²	2½	6-10d	2-10dx1½	-	2935	2935	LSU5.12 ³	31⁄2	24-16d	16-10dx1½	910	2600	1835
14	THAI-2 ²	21/2	6-10d	2-10dx1½	—	2935	2935	LSU5.12 ³	3½	24-16d	16-10dx1½	910	2600	1835
16	See Cana	dian \		struction Co Iger selectio		rs Cata	logue	See Cana	dian W		struction Co ger selectior		s Catal	ogue
Double	e AJS 25					Joist	Width	= 7"						
9½														
11%														
14	See Cana	dian \		struction Co Iger selectio		rs Cata	logue	See Cana	dian W		struction Co ger selectior		s Catal	ogue
16			101 1101	iger selectio						ior nur	ger selection			
18														

- 1. Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by others.
- THAI hangers require a minimum of 4 top and 2 face nails 2. installed. THAI-2 must be special ordered, specify hanger seat width between 3½," and $5^{5}\!/_{16}$ "
- LSU5.12 skew options must be factory ordered.
- 4. Skewed option must be special ordered. Specify skew angle and direction (i.e. HU410-2X, SKR 45°).
- 5. The B Dim is the depth of the hanger seat.

Double AJS® 25 (Joist Width = 7") Adjustable Height and Field Slope & Skew Hangers - See Wood Construction Connectors Catalogue for hanger selection. ALLJOIST® Specifier Guide - CANADA November 2011

Framing Connectors - Simpson Strong-Tie

General Notes

- See current Canadian Wood Construction Connectors catalogue for Important Information and General Notes section and for hanger models, joist sizes, and header situations not shown. See pages 10-12 of the Simpson Strong-Tie Publication CSG-BCCAN08 " AJS I-JOISTS Connector Selection Guide, Limit States Design" version for installation information.
- 2. Unless otherwise noted, factored resistances (downloads) listed address hanger/header/fastener limitations assuming header material is Douglas Fir-Larch or Spruce Pine Fir. For LVL headers made primarily of Douglas Fir/Southern Pine, use the values found in the DF column. For LVL headers made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the SPF column. Loads are in pounds. Joist reaction should be checked by a qualified designer to ensure proper hanger selection.
- Factored uplift resistances (uplift) listed assume SPF joist and header and have been increased by 15% for earthquake and wind loading with no further increase allowed. Reduce loads according to code for normal duration loading such as cantilever construction.
- If hanger height is less than 60% of joist height, joist rotation may occur; see Prevent Rotation information on page 2 of the Simpson guide referenced in Note 1 above.

HOW TO PICK A HANGER:

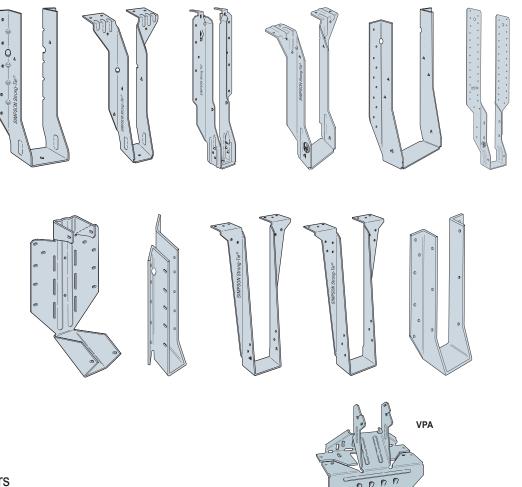
- 1. Find your joist size in this guide.
- 2. Choose your header type. Solid header or I-joist.
 - Solid headers include solid sawn
 Douglas Fir, Spruce-Pine Fir, and LVL.
 - For I-joist header see page 2 of the guide referred to in Note 1 of General Notes.
- 3. Locate your connector type in the table.
 - Face mount, top flange, skewed, sloped, etc.
- 4. Select a hanger from the table.
- Confirm that your factored joist reaction is less than the factored resistance of hanger.
 If yes, you have successfully selected your hanger.

If you did not find a suitable hanger; please see the current Canadian Wood Construction Connectors catalogue or call Simpson Strong-Tie at 800-999-5099.

You will need the following information:

- Download
- Uplift
- Header condition
- · Bearing length requirement

- 5. Top flange hanger configuration and thickness of top flange need to be considered for flush frame conditions, see page 10 of the Simpson guide referenced in Note 1 to the left.
- 6. For this publication, carrying members are assumed to be at least 5½ inches tall. The horizontal thickness of the carrying member must be at least the length of nail being used or the hanger top flange dimension, whichever is greater. Exception: narrower carrying members may be used with face mount hangers but the horizontal thickness must be at least 1% inches for 10d nails; 2 inches for 16d nails. Clinch nails on back side.
- THAI hangers in this publication are based on a "top flange" installation and require that the carrying member have a horizontal thickness of at least 2½ inches. Backer blocks are required when the header is an I-joist.
- 8. All nails shown are common nails unless otherwise noted.
- 9. I-joists that are used as headers require backer blocks. See Wood I-Joist Headers in the Simpson Strong-Tie guide referenced in Note 1 for additional information.
- 10. **Multiple Members:** Multiple members should be adequately connected together to act as one unit.



VPA - Variable Pitch Connectors

				Factored Resistances												
		Fast	eners			Fa	ctored R	esistan	ces							
Joist	Model		Uplift Down Load		n Load		Lateral	[115%]								
Width	No.	Top Plate	Rafter	[115%]		[10	[100]		DF	SPF						
				DF	SPF	DF	SPF	F1	F_2	F1	F_2					
21⁄2	VPA3	9-10d	2-10dx1½	545	390	1785	1785	625	415	615	415					
31/2	VPA4	11-10d	2-10dx1½	545	390	1785	1785	625	415	615	415					

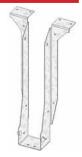
- This variable pitch connector allows a sloped beam to sit on a top plate without having to notch, birdmouth, bevel, or toe nail. It also provides uplift capacity. Adjustable from 3:12 to 12:12 pitch.
- VPAs are not appropriate for applications that require more than 2" of bearing, such as intermediate supports.

SINCLE I- INISTS - Canadian/Factored Posistance (lbs)

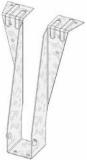
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			Top Mount Han							Face Mount Har	ř.			
		Fastene	er Schedule ⁴	DF-	L/SP		P-F		Fasten	er Schedule⁴	DF-L			P-F
oist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
ight	Stock No. ^{1,5}	Header	Joist	100%	115%	100%	115%	Stock No. ¹	Header	Joist	100%	115%	100%	115%
JS™				0.10.5				= 2-1/2″	(10) (0)		00/0			
·1/2	TFL2595	(6) 10d	(2) 10d x 1-1/2	2495	745	1771	530	THF25925	(12) 10d	(2) 10d x 1-1/2	3310	335	2350	238
-7/8 14	TFL25118 TFL2514	(6) 10d	(2) 10d x 1-1/2	2495	745	1771	530	THF25112 THF25140	(14) 10d	(2) 10d x 1-1/2	3310	720	2350	511
14	TFL2514 TFL2516	(6) 10d	(2) 10d x 1-1/2	2495 2495	745 745	1771 1771	530 530	THF25140 THF25160	(18) 10d	(2) 10d x 1-1/2	4405 4405	720 720	3128 3128	511 511
JS™		(6) 10d	(2) 10d x 1-1/2	2495	743		Width =		(22) 10d	(2) 10d x 1-1/2	4400	720	3120	511
-1/2	THO35950	(10) 10d	(2) 10d x 1-1/2	2975	500	2115	355	- 3-1/2 THF35925	(12) 10d	(2) 10d x 1-1/2	5240	465	3720	330
1-7/8	THO35330	(10) 10d	(2) 10d x 1-1/2 (2) 10d x 1-1/2	2975	500	2115	355	THF35112	(12) 10d	(2) 10d x 1-1/2 (2) 10d x 1-1/2	5240	465	3720	330
14	THO35140	(10) 10d	(2) 10d x 1-1/2 (2) 10d x 1-1/2	4450	500	3160	355	THF35140	(10) 10d	(2) 10d x 1-1/2	6680	465	4743	330
16	THO35160	(12) 10d	(2) 10d x 1-1/2	4450	500	3160	355	THF35157	(22) 10d	(2) 10d x 1-1/2	6680	465	4743	330
18	TFI418	(6) 16d	(2) 10d x 1-1/2	4190	545	2975	385	THF35157	(22) 10d	(2) 10d x 1-1/2	6680	465	4743	330
			justable Height I	Hangers						Slope and Skew H	angers			
		-	er Schedule ⁴		L/SP	S-	P-F		-	er Schedule⁴	DF-L	/SP	S-I	P-F
oist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
eight	Stock No. ^{1,6}	Header	Joist	100%	115%	100%	115%	Stock No. ¹	Header	Joist	100%	115%	100%	115%
Js™	' 140 or AJS™						Width :							
-1/2								LSSH25	(14) 16d	(12) 10d x 1-1/2	4260	1955	3025	1390
1-7/8	MSH318	(6) 10d	(4) 10d x 1-1/2	1185		840		LSSH25	(14) 16d	(12) 10d x 1-1/2	4260	1955	3025	1390
14	MSH318	(6) 10d	(4) 10d x 1-1/2	1185		840		LSSH25	(14) 16d	(12) 10d x 1-1/2	4260	1955	3025	1390
16	MSH318	(6) 10d	(4) 10d x 1-1/2	1185		840		LSSH258	(14) 16d	(12) 10d x 1-1/2	4260	1955	3025	1390
∖JS™	25					Joist	Width =	= 3-1/2″						
9-1/2	MSH422	(6) 10d	(4) 10d	1105		785		LSSH35	(14) 16d	(12) 10d x 1-1/2	5230	2595	3715	1845
1-7/8	MSH422	(6) 10d	(4) 10d	1105		785		LSSH35	(14) 16d	(12) 10d x 1-1/2	5230	2595	3715	1845
14								LSSH35	(14) 16d	(12) 10d x 1-1/2	5230	2595	3715	1845
16								LSSH35 ⁸	(14) 16d	(12) 10d x 1-1/2	5230	2595	3715	1845
18								LSSH35 ⁸	(14) 16d	(12) 10d x 1-1/2	5230	2595	3715	1845
			Skewed 45° Ha											
		Fastene	er Schedule⁴		L/SP		P-F	,	•	web stiffeners at joi			ners may	be
oist eight	USP Stock No.1			Down ²	Uplift ³	Down ²	Uplift ³	required for	hop choded	hangers by I-joist m	anufactur	ers.		
eigin					4480/	4000/	4450/			0004 NID00				0.0
		Plate	Rafter	100%	115%	100%	115%	2) Loads liste	d are based o	n 2001 NDS® and h				
	' 140 or AJS™	20	Joist	Width	= 2-1/2″			2) Loads lister or S-P-F sp	d are based of recies solid sa	wn or LVL header.	Some load	ds may b	e increas	ed for
9-1/2	140 or AJS™ SKH2520L/R	20 (14) 10d	Joist (10) 10d x 1-1/2	Width = 3265	= 2-1/2 " 2910	2320	2065	2) Loads lister or S-P-F sp duration of	d are based of ecies solid sa load adjustme	wn or LVL header. ents. Refer to USP I	Some load Full Line (ds may b Catalog fo	e increas or details.	ed for
9-1/2 1-7/8	140 or AJS™ SKH2520L/R SKH2520L/R	20 (14) 10d (14) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2	Width = 3265 3265	= 2-1/2 ~ 2910 2910	2320 2320	2065 2065	2) Loads lister or S-P-F sp duration of 3) Uplift loads	d are based of ecies solid sa load adjustme have been in	wn or LVL header. ents. Refer to USP I creased 15% for wir	Some load Full Line (ds may b Catalog fo	e increas or details.	ed for
9-1/2 1-7/8 14	140 or AJS™ SKH2520L/R SKH2520L/R SKH2524L/R	20 (14) 10d (14) 10d (16) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2	Width = 3265 3265 3265	= 2-1/2 ["] 2910 2910 2910	2320 2320 2320	2065 2065 2065	 Loads lister or S-P-F sp duration of Uplift loads increase sl 	d are based o ecies solid sa load adjustm have been in nall be permitt	wn or LVL header. S ents. Refer to USP I creased 15% for wir ted.	Some load Full Line (nd and sei	ds may b Catalog fo ismic load	e increas or details.	ed for
16	4 140 or AJS™ SKH2520L/R SKH2520L/R SKH2524L/R SKH2524L/R	20 (14) 10d (14) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2	Width 3265 3265 3265 3265	= 2-1/2 ^{°°} 2910 2910 2910 2910	2320 2320	2065 2065	 Loads lister or S-P-F sp duration of Uplift loads increase sl 10d x 1-1/2 	d are based o lecies solid sa load adjustm have been in hall be permitt ″ nails are 9 g	wn or LVL header. S ents. Refer to USP I creased 15% for wir ted. Jauge (0.148" diame	Some load Full Line C nd and sei ter) by 1-'	ds may b Catalog fo ismic load	e increas or details.	ed for
9-1/2 1-7/8 14	4 140 or AJS™ SKH2520L/R SKH2520L/R SKH2524L/R SKH2524L/R	20 (14) 10d (14) 10d (16) 10d (16) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 Joist	Width = 3265 3265 3265	= 2-1/2 ^{°°} 2910 2910 2910 2910	2320 2320 2320	2065 2065 2065	 Loads lister or S-P-F sp duration of Uplift loads increase sl 10d x 1-1/2 Minimum n 	d are based o ecies solid sa load adjustm have been in hall be permitt " nails are 9 g ail penetration	wn or LVL header. S ents. Refer to USP I creased 15% for wir ted.	Some load Full Line C nd and sei ter) by 1- ⁻ 10d nails.	ds may b Catalog fo ismic load 1/2″ long.	e increas or details. ding; no fi	ed for urther
9-1/2 1-7/8 14 16 AJS™ 9-1/2	4 140 or AJS M SKH2520L/R SKH2520L/R SKH2524L/R SKH2524L/R 25	20 (14) 10d (14) 10d (16) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2	Width = 3265 3265 3265 3265 Width =	= 2-1/2 ^{°°} 2910 2910 2910 2910 = 3-1/2 ^{°°}	2320 2320 2320 2320	2065 2065 2065 2065	 2) Loads lister or S-P-F sp duration of 3) Uplift loads increase si 4) 10d x 1-1/2 Minimum n 16d sinkers 	d are based o ecies solid sa load adjustm have been in hall be permitt nails are 9 g ail penetration (0.148" diam	wn or LVL header. S ents. Refer to USP I creased 15% for wir ted. lauge (0.148" diame a shall be 1-1/2" for 1	Some load Full Line C nd and sei ter) by 1- ⁻ 10d nails.	ds may b Catalog fo ismic load 1/2″ long.	e increas or details. ding; no fi	ed for urther
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9-1/2 1-7/8 14 16 -1/2 1-7/8 14 16 1-7/8 14 16 18 0ist /idth AJS T 2-1/2 -1/2 	140 or AJS™ SKH2520L/R SKH2520L/R SKH2524L/R SKH2524L/R 25 SKH410L/R' SKH410L/R' SKH414L/R'	20 (14) 10d (14) 10d (16) 10d (16) 10d (16) 16d (16) 16d (22) 16d (20) 10d (20) 10d	Joist (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 (10) 10d x 1-1/2 Joist (10) 16d (10)	Width 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3265 3690 7405 7405 7405 7405 7405 00wn² Down² 100% 2630 3485 2835 3485	 2-1/2" 2910 2910 2910 2910 3560 3560 3560 3560 3560 400 <l< td=""><td>2320 2320 2320 2320 2620 2620 5260 5260 5260 5260 5260 52</td><td>2065 2065 2065 2530 2530 2530 2530 2530 2530 2530 253</td><td> 2) Loads lister or S-P-F sp duration of 3) Uplift loads increase sl 4) 10d x 1-1/2 Minimum n 16d sinkers with no load 5) Top Mount 3-1/2" minir 6) For addition refer to USI 7) Miter cut re 8) LSTA24 str For further please cal or go to www.USPc </td><td>d are based o eccies solid sa load adjustmu have been in nall be permitt " nails are 9 g ail penetration (0.148" diam d reduction. Hangers requind num header th nal sizes, stoc P's Full Line C quired on end ap required al er informa 1.1.800.32</td><td>wn or LVL header. 3 ents. Refer to USP I creased 15% for wir led. Is aluge (0.148" diame Is shall be 1-1/2" for eter) by 3-1/4" long uire minimum 3" hea hickness for all othe k numbers, and mor Catalog. It of joist to achieve of long top chord for la tion 28.5934</td><td>Some load Full Line C nd and sei ter) by 1-' 10d nails. may be su der width r stock nu difications design loa</td><td>ds may b Catalog fc ismic load 1/2" long. ubstituted for THO imbers. a not show ds.</td><td>e increas or details. ding; no fi I for 10d d series ha</td><td>ed for urther common nai nge</td></l<>	2320 2320 2320 2320 2620 2620 5260 5260 5260 5260 5260 52	2065 2065 2065 2530 2530 2530 2530 2530 2530 2530 253	 2) Loads lister or S-P-F sp duration of 3) Uplift loads increase sl 4) 10d x 1-1/2 Minimum n 16d sinkers with no load 5) Top Mount 3-1/2" minir 6) For addition refer to USI 7) Miter cut re 8) LSTA24 str For further please cal or go to www.USPc 	d are based o eccies solid sa load adjustmu have been in nall be permitt " nails are 9 g ail penetration (0.148" diam d reduction. Hangers requind num header th nal sizes, stoc P's Full Line C quired on end ap required al er informa 1.1.800.32	wn or LVL header. 3 ents. Refer to USP I creased 15% for wir led. Is aluge (0.148" diame Is shall be 1-1/2" for eter) by 3-1/4" long uire minimum 3" hea hickness for all othe k numbers, and mor Catalog. It of joist to achieve of long top chord for la tion 28.5934	Some load Full Line C nd and sei ter) by 1-' 10d nails. may be su der width r stock nu difications design loa	ds may b Catalog fc ismic load 1/2" long. ubstituted for THO imbers. a not show ds.	e increas or details. ding; no fi I for 10d d series ha	ed for urther common nai nge



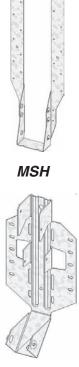
TFL



тно



THF single



TMPH

4) 10d x 1-1/2" nails are 9 gauge (0.148" diameter) by 1-1/2" long. Minimum nail penetration shall be 1-1/2" for 10d nails.

3) Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted.

DOUBLE I-JOISTS - Canadian/Factored Resistance (lbs)



		Т	op Mount Ha	ngers⁵					Fa	ce Mount Han	igers			
		Fastener	Schedule ⁴	DF-L	_/SP	S-I	P-F		Fastener	Schedule ⁴	DF-L	_/SP	S-I	P-F
Joist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
Height	Stock No. 1,5	Header	Joist	100%	115%	100%	115%	Stock No.1	Header	Joist	100%	115%	100%	115%
AJS™	140 or AJS™	20				Jois	st Width	= 5″						
9-1/2	THO25950-2	(10) 16d	(6) 10d	6005	2210	4265	1570	THF25925-2	(12) 10d	(6) 10d	5240	3325	3720	2361
11-7/8	THO25118-2	(10) 16d	(6) 10d	6005	2210	4265	1570	THF25112-2	(16) 10d	(6) 10d	5240	3325	3720	2361
14	THO25140-2	(12) 16d	(6) 10d	6645	2210	4715	1570	THF25140-2	(20) 10d	(6) 10d	6680	3325	4743	2361
16	THO25160-2	(12) 16d	(6) 10d	6645	2210	4715	1570	THF25160-2	(24) 10d	(6) 10d	6680	3325	4743	2361
18	THO25180-2	(14) 16d	(6) 10d	9500	2210	6745	1570	THF25160-2	(24) 10d	(6) 10d	6680	3325	4743	2361
AJS™	25					Jois	st Width	ı = 7″						
9-1/2	BPH7195	(10) 16d	(6) 10d	5055	1245	4725	885	HD7100	(12) 16d	(6) 10d	7215	4435	5123	3149
11-7/8	BPH71118	(10) 16d	(6) 10d	5055	1245	4725	885	HD7120	(16) 16d	(6) 10d	7215	4435	5123	3149
14	BPH7114	(10) 16d	(6) 10d	5055	1245	4725	885	HD7120	(16) 16d	(6) 10d	7215	4435	5123	3149
16	BPH7116	(10) 16d	(6) 10d	5055	1245	4725	885	HD7160	(24) 16d	(8) 10d	7215	4435	5123	3149
18	BPH7118	(10) 16d	(6) 10d	5055	1245	4725	885	HD7160	(24) 16d	(8) 10d	7215	4435	5123	3149
		Adju	istable Height	Hangers					Sk	ewed 45° Har	igers			
		Fastener	Schedule ⁴	DF-L		S-I	P-F		Fastener	Schedule ⁴	DF-l	_/SP	S-I	P-F
Joist	USP			Down ²	Uplift ³	Down ²	Uplift ³	USP			Down ²	Uplift ³	Down ²	Uplift ³
Height	Stock No. 1,8	Header	Joist	100%	115%	100%	115%	Stock No.1	Plate	Rafter	100%	115%	100%	115%
AJS™	140 or AJS™	20				Jois	st Width	= 5″						
9-1/2	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2520L/R-27	(14) 10d	(10) 10d	5430	3565	3855	2530
11-7/8	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
14	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
16	MSH2622-2	(6) 10d	(4) 10d	1490		1055		SKH2524L/R-27	(16) 10d	(10) 10d	5055	3560	3590	2530
AJS™	25					Jois	st Width	ı = 7″						
9-1/2								HD7100-SK45L/R ^{6,7}	(12) 16d	(6) 10d	7215	3325	5123	2360
11-7/8	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7120-SK45L/R ^{6,7}	(16) 16d	(6) 10d	7215	3325	5123	2360
14	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7120-SK45L/R ^{6,7}	(16) 16d	(6) 10d	7215	3325	5123	2360
16	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7160-SK45L/R ^{6,7}	(24) 16d	(8) 10d	7215	3325	5123	2360
18	MSH422-2	(8) 16d	(6) 16d	2295		1630		HD7160-SK45L/R ^{6,7}	(24) 16d	(8) 10d	7215	3325	5123	2360

1) Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required for non-shaded hangers by I-joist manufacturers. 2) Loads listed are based on 2001 NDS® and hanger attachment to a DF-L, SP, or S-P-F species solid sawn or LVL header.

Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details.

3) Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted.

4) 10d x 1-1/2" nails are 9 gauge (0.148"" diameter) by 1-1/2" long.

Minimum nail penetration shall be 1-1/2" for 10d nails and 1-5/8" for 16d nails.

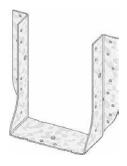
16d sinkers (0.148" diameter) by 3-1/4" long may be substituted for 10d common nails with no load reduction.

5) Top Mount Hangers require minimum 3" header width for THO series hangers; 3-1/2" minimum header thickness for all other stock numbers.

6) Hangers are special order. Consult USP for pricing and lead times.

7) Miter cut required on end of joist to achieve design loads.

8) For additional sizes, stock numbers, and modifications not shown, refer to USP's Full Line Catalog.





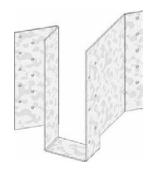
BPH



THF Double

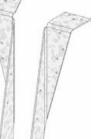


THO Double



SKH L Double left shown

HD



For further information

please call 1.800.328.5934

or go to www.USPconnectors.com

Computer Software

BC FRAMER[®] CADD Framing Software

BC FRAMER[®] is an easy-to-use, stand-alone computer-aided drafting program with 3D capability designed to quickly create floor and roof framing layouts.

Draw walls, add framing areas, locate beams and rough openings, define cross-sections and on-center spacing. In minutes, BC FRAMER® frames your layout, builds a piece and price report, creates a framing drawing with schedule, all using Boise Cascade Engineered Wood Products.

Easy-to-use editing and drawing tools allow flexibility when modifying members and adding details, symbols and accessories.

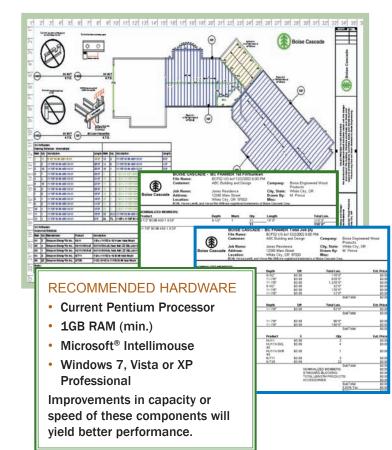
Training classes for BC FRAMER[®] are provided at Boise Cascade training centers located in Oregon, Louisiana and Georgia.

For questions, comments or a free demonstration CD visit our website at www.BC.com/ewp or email us at EWPSupport@BC.com. Information can also be obtained at 800-405-5969.

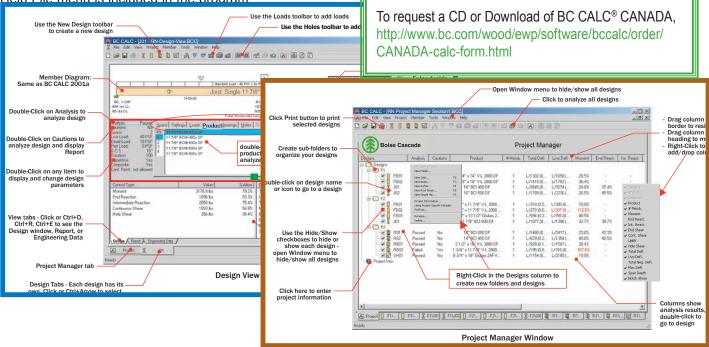
BC CALC® Sizing Software

BC CALC[®] is Boise Cascade's sizing software for AJS[®] Joists and VERSA-LAM[®] Beams.

BC CALC[®] is simple to use, yet flexible enough to analyze most joist and beam applications. Span, load and hole information entered by the user is analyzed by the program to correctly size Boise Cascade Engineered Wood Products. A comprehensive Help File menu is included in the program.



BC CALC[®] is available to designers, architects and engineers on CD-ROM or by download at www.BC.com/ewp. Hardware requirements include Pentium[®] Processor with a recommended minimum of 512MB RAM. To order BC CALC[®], call 800-405-5969, email EWPSupport@ BC.com or visit our website at www.bc.com/ewp/software.



Weights of Building Materials

CEILING Pounds Per Square	
Acoustical fiber tile (1)	1
Suspended steel channel system ⁽¹⁾	2
Suspended wood channel system	2.5
2x8 ceiling joists @ 16" o.c., R-49 insulation,	7
1/2" gypsum board	0
1" Plaster	8
1/2" gypsum board	2.2
5/8" gypsum board	2.75
ROOF Pounds Per Square	
Fiberglass shingles	3
Asphalt shingles ⁽¹⁾	2
Wood shingles ⁽¹⁾	3
Spanish clay tile (1)	19
Composition Roofing:	
Three-ply ready roofing ⁽¹⁾	1
Four-ply felt and gravel ⁽¹⁾	5.5
Five-ply felt and gravel (1)	6
20 gage metal deck ⁽¹⁾	2.5
18 gage metal deck ⁽¹⁾	3
1" fiberglass batt insulation	0.04
1" loose fiberglass insulation	0.04
1" loose cellulose insulation	0.14
1" rigid insulation ⁽¹⁾	1.5
³ /16" slate ⁽¹⁾	7
1/4" slate (1)	10
Single-ply (no ballast) ⁽¹⁾	0.7
Single-ply (ballasted)	11
Dry gravel (1)	8.7
2x8 rafters @ 16" o.c., fiberglass shingles, 15# felt,	8
%" sheathing	
Skylight: metal frame w/ ¾" wire glass (1)	8
FLOOR Pounds Per Square	
1" reinforced regular weight concrete	12.5
1" plain lightweight concrete ⁽¹⁾	8
$7/_{16}$ " cementitious backerboard Ceramic or quarry tile ($3/_4$ ") on $1/_2$ " mortar bed $^{(1)}$	3 16
Ceramic of quarry tile $(\frac{3}{4})$ on 1° mortar bed $(^{\circ})$	23
1" mortar bed	12
1" slate ⁽¹⁾	12
%" marble tile	6
%" ceramic floor tile ⁽¹⁾	4.7
Hardwood flooring, 7/7-in ⁽¹⁾	4
1/4" linoleum or asphalt tile (1)	1
BCI®/AJS® joists @ 16" o.c., 3/4" sheathing, 1/2" gypsum board	10
3/4" Gyp-Crete topping	6.5
Carpet & Pad	2.0
Waterproofing Membranes	
Bituminous, smooth surface ⁽¹⁾	1.5
Liquid applied ⁽¹⁾	1
MISCELLANEOUS Pounds Per Square	Foot [PSF]
1" of sand	8
1" of water	5.2
Hay: baled, dry ⁽²⁾	15 PSF ⁽²⁾
Straw: baled, dry ⁽²⁾	8 PSF ⁽²⁾
Saturated soil (garden/landscaped roof)	135 PCF
Grand piano	1000 LBS

(1) Minimum Design Loads for Buildings and Other Structures, ASCE 7-05.

(2) National Farm Building Code (Canada) 1995. Value in pounds per cubic foot (PCF), multiply by maximum height to obtain PSF.

SHEATHING Pounds Per Square	Foot [PSF]
¹¹ / ₃₂ " or 3/8" Plywood – OSB ⁽³⁾	1.0 – 1.2
¹⁵ / ₃₂ " or ¹ / ₂ " Plywood – OSB ⁽³⁾	1.4 – 1.7
¹⁹ / ₃₂ " or ⁵ / ₈ " Plywood – OSB ⁽³⁾	1.8 – 2.1
²³ / ₃₂ " or ³ / ₄ " Plywood – OSB ⁽³⁾	2.2 – 2.5
7/8" Plywood – OSB ⁽³⁾	2.6 – 2.9
1 ¹ / ₈ " Plywood – OSB ⁽³⁾	3.3 – 3.6
1/2" cementitious backerboard	3
1 ¹ / ₂ " softwood T & G decking	4.6
FLOOR FRAMING Pounds Per Square	Foot [PSF]
2x4 @ 16" o.c.	1.1
2x6 @ 16" o.c.	1.7
2x8 @ 16" o.c.	2.2
2x10 @ 16" o.c.	2.9
2x12 @ 16" o.c.	3.5
BCI® 4500s, 5000 or 5000s @ 12" o.c.	2.1 – 2.9
BCI [®] 4500s, 5000 or 5000s @ 16" o.c.	1.6 – 2.2
BCI [®] 4500s, 5000 or 5000s @ 19.2" o.c.	1.3 – 1.8
BCI [®] 4500s, 5000 or 5000s @ 24" o.c.	1.1 – 1.5
BCI® 6000 or 6000s @ 12" o.c.	2.5 – 3.4
BCI [®] 6000 or 6000s @ 16" o.c.	1.9 – 2.6
BCI® 6000 or 6000s @19.2" o.c.	1.6 - 2.1
BCI® 6000 or 6000s @ 24" o.c.	1.3 - 1.7
BCI® 60, 60s, 6500 or 6500s @ 12" o.c.	2.5 – 3.8
BCI® 60, 60s, 6500 or 6500s @ 16" o.c.	1.9 - 2.9
BCI® 60, 60s, 6000 or 6500s @19.2" o.c.	1.6 – 2.4
BCI® 60, 60s, 6500 or 6500s @ 24" o.c.	1.3 – 1.9
BCI® 90, 90s or 90e @ 12" o.c.	3.9 - 5.4
BCI® 90, 90s or 90e @ 16" o.c.	2.9 - 4.1
BCI® 90, 90s or 90e @ 19.2" o.c.	2.3 - 4.1
BCI® 90, 90s or 90e @ 24" o.c.	1.9 – 2.7
AJS [®] 140 or 20 @ 12" o.c.	1.3 - 2.7 2.2 - 3.3
AJS® 140 or 20 @ 16" o.c.	1.7 – 2.5
AJS [®] 140 or 20 @ 19.2" o.c.	1.4 - 2.1
AJS [®] 140 or 20 @ 24" o.c.	1.1 – 1.7
AJS [®] 25 @ 12" o.c.	3.1 – 5.4
AJS [®] 25 @ 16" o.c.	2.3 – 4.1
AJS [®] 25 @ 19.2" o.c.	1.9 – 3.4
AJS [®] 25 @ 24" o.c.	1.6 – 2.7
WALL Pounds Per Square	
⁵ / ₁₆ " x 7 ¹ / ₂ " fiber cement lap siding	3
4" clay brick ⁽¹⁾	39
1/4" ceramic wall tile (1)	3.1
1¾" Cultured Stone	12
2x4 studs @ 16" o.c., 5%" gypsum, insulation, 3%" siding (1)	11
2x6 studs @ 16" o.c., 5%" gypsum, insulation, 3%" siding ⁽¹⁾	12
Wood or steel studs, ¹ / ₂ " gypsum board each side ⁽¹⁾	8
Exterior stud walls w/ brick veneer ⁽¹⁾	48
Stucco	10
Log Wall: 10" diameter	26
Glass Block:	20
4" Thick - standard (hollow)	20
3" Thick - standard (hollow)	16
4" Thick - thin face	30
3" Thick - solid glass block	40
Windows: glass, frame and sash ⁽¹⁾	8
Include at least 1.5 psf in all dead load summations t for incidentals such as plumbing, ducts, light fixtures	o account , etc.

(3) Approximate Engineering Dead Load Weight of Wood Structural Panels, APA EWS TT-019, 2005.



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